



Skevington, E., & Hogg, A. J. (2019). Unsteady draining of reservoirs over weirs and through constrictions. *Journal of Fluid Mechanics*, 882, [A9]. <https://doi.org/10.1017/jfm.2019.808>

Peer reviewed version

Link to published version (if available):
[10.1017/jfm.2019.808](https://doi.org/10.1017/jfm.2019.808)

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Unsteady draining of reservoirs over weirs and through constrictions

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(Received xx; revised xx; accepted xx)

The gravitationally-driven flow of fluid from a reservoir following the partial collapse of its confining dam, or the partial opening of its confining lock, is modelled using the nonlinear shallow water equations, coupled to outflow conditions, in which the drainage is modelled as flow over a constricted, broad-crested weir. The resulting unsteady motion reveals systematic draining, on which strong and relatively rapid oscillations are imposed. The oscillations propagate between the outflow and the impermeable back wall of the reservoir. These dynamics are investigated utilising three methods: hodograph techniques to yield quasi-analytical solutions, asymptotic analysis at relatively late times after initiation, and numerical integration of the governing equations. The hodograph transformation is used to find exact solutions at early times, revealing that from initially quiescent conditions the fluid drains and yet repeatedly generates intervals during which there are regions of constant depth and velocity adjacent to the boundaries. A novel modified multiscale asymptotic analysis designed for late times is employed to determine the limiting rate of draining and wave structure. It is shown that the excess height drains as t^{-2} and, when the obstacle has finite height, the velocity field decays as t^{-3} , and exhibits a wave structure that tends towards a constant and relatively rapid phase speed. In the case of a pure constriction, for which all the fluid ultimately drains out of the reservoir, the motion adjusts to a self-similar state in which the velocity field decays as t^{-1} . Oscillations around this state have an exponentially increasing period. Numerical simulations with a novel implementation of boundary conditions are performed; they confirm the hodograph solution and provide data for the asymptotic results.

1. Introduction

Understanding the partial or total collapse of dams and the resulting fluid flow is vital for the protection of people, infrastructure, and environment downstream. When a dam fails, it is common for only a portion of it to collapse, creating an obstacle for the upstream reservoir to navigate before it rushes downstream. This results in a gravity current, where the horizontal density difference between the current and ambient fluid drives the flow (Simpson 1997; Ungarish 2009). The investigation into gravity currents has a long history (von Kármán 1940) due to their many industrial and environmental applications, including open channel hydraulics, ventilation, the spreading of toxic gas, intrusion of saline water in estuaries, and chemical and oil spillage. Important fundamental problems in this research area include ‘dam-break’ and ‘lock-release’ flows, in which initially quiescent fluid held behind a barrier, surrounded by a lower density ambient, is suddenly released to flow along a channel. The term ‘lock-release’ usually relates to Boussinesq flows for which the density ratio between the current and ambient is order unity, whereas the term ‘dam-break’ is often used for non-Boussinesq flows in this configuration, importantly including the subaerial collapse of a fluid reservoir.

These flows are not only important for their applications, but are well established experiments in laboratory settings as documented by Simpson (1982) and Ungarish (2009), including flows interacting with barriers (Lane-Serff *et al.* 1995; Wilson *et al.* 2017). The motion is non-trivial because the dynamics evolve spatially and temporally, and yet the configuration is sufficiently simple to allow the fundamental fluid mechanical mechanisms to be clearly investigated.

When modelling gravity currents and other free-surface hydraulic flows, their extreme aspect ratio is exploited, as the motion typically has vastly greater horizontal extent than depth. This asymptotic regime gives rise to a hydrostatic pressure field, and thence the shallow water equations can be deduced (Ungarish 2009). Around the flow head where the current propagates into the ambient, however, hydrostatic pressure is not an accurate approximation, and special considerations must be made. Benjamin (1968) performed one of the first comprehensive investigations into the steady dynamics of the head for two layer flows in a lidded channel, and established conditions of the form $u = Fr\sqrt{gh}$ relating the velocity of head u to gravitational acceleration g and the depth of upstream flow h by the Froude number Fr , where the Froude number is a function of both the ratio of the flow depth to the channel height, and the ratio of the current to ambient densities. This work has been recently revisited by Ungarish & Hogg (2018) to account for the vorticity developed by the flow as it overpasses the front. Solutions for unsteady dam-break and lock-release flows have been constructed by employing the Froude number condition as a dynamical boundary condition for the shallow water equations at the front of the motion (Rottman & Simpson 1983). From lock release initial conditions, the current rapidly accelerates to propagate with constant velocity (Ungarish 2009) before entering a self-similar regime in which the inertia of the fluid motion is in balance with the hydrostatic pressure gradients (Fannelop & Waldman 1972; Hoult 1972). The approach to self-similarity is characterised by interfacial waves that propagate between the front of the current and the backwall of the reservoir (Hogg 2006).

Despite the extensive investigation into dam-break flows, there are many important variations that have not been studied. In this work we consider the possibility of incomplete breakage of a dam, either by collapse of the upper portions, creating a weir, or the centre portion, creating a constriction. A similar situation occurs with the sudden (partial) opening of a sluice gate in a canal. Broad-crested weirs (BCW), where the horizontal crest extends for a sufficient length such that critical flow with hydrostatic pressure is established (Chow 2009), are of particular interest here. Early work on this topic was documented by Horton (1907) for sharp-edged BCW, in which the flux of fluid over the weir is related by empirical relationships to upstream flow characteristics. Whilst many different empirical relationships are still in use, the one presented Ackers *et al.* (1978) is useful for conceptual understanding as it uses a drainage coefficient C_d which can be related to the ratio of upstream energy-head H_u to crest energy-head H_c (using the crest height as the datum) as $H_c/H_u = C_d^{2/3}$. Values of C_d are calculated from data compiled from eighteen different studies in Zachoval *et al.* (2014) and from their results we calculate a head loss of 3 to 12%. Experimental results for a variety of ramped BCW are presented in Azimi *et al.* (2013) and, whilst their choice of drainage coefficient is somewhat different to that of Zachoval *et al.* (2014), they find even lower head losses. This indicates that broad crested weirs may be modelled to leading order by a shallow water model with the overflow represented by a condition that enforces the conservation of energy between the bottom and top of the weir, with critical flow at the top. This is the approach taken by Lane-Serff *et al.* (1995), Karelskii & Petrosyan (2006) and Valiani & Caleffi (2017) for both ramped and sharp-edged BCW.

A similar variation on the dam break was presented by Momen *et al.* (2017), where the dam-break occurred at a vertical edge so that critical flow was rapidly established. Both experiments and direct numerical simulations of the Navier Stokes equations were presented, and compared to numerical solutions of the shallow water equations and an analytical similarity solution that emerges after a sufficient time following flow initiation. Close correspondence was found between the modelling and experimental results. The present work may be considered both a generalisation and extension of their shallow water analysis: a generalisation because we include the possibility of a weir and/or constriction between the bulk and the critical flow, and an extension because we employ a variety of additional analytical and numerical techniques to investigate the dynamics.

Flows modelled by the nonlinear shallow water equations may be calculated using quasi-analytical techniques when analysed in terms of hodograph variables (see, for example, Hogg (2006)) and this approach is used in this study to reveal the initial phases of the motion as the collapsing reservoir of fluid interacts with the containing barrier or constriction. Hodograph techniques for hyperbolic partial differential equations, such as the shallow water equations, interchange the dependent and independent variables and it is then convenient to treat the space and time variables as functions of the two characteristics variables of the shallow water equations. Under this transformation the governing equations become linear and may then be solved using an appropriate Green's function; the variables are determined as integrals that can be evaluated easily with elementary numerical techniques to high numerical precision. This quasi-analytical approach has advantages in that it is easy to track the locations of positions where the values or gradients of the original variables change discontinuously and it is straightforward to calculate the spatial and temporal domains over which the confining barrier influences the flow. The hodograph technique has been deployed in a number of circumstances including lock-release gravity currents (Hogg 2006), dam-break collapse into a channel with a pre-existing fluid layer (Goater & Hogg 2011), swash evolution and bore formation (Antuono & Hogg 2009) and flow run-up and overtopping sloping beaches (Hogg *et al.* 2011). In this study we will show from the hodograph analysis that the flow generates waves that propagate between the end wall of the lock and the barrier or constriction, while the fluid unsteadily drains. These oscillations lead to successive intervals during which the flow depth is locally horizontal (and the velocity uniform) at either end of the domain. These oscillations persist until the depth of the fluid reduces to the height of the barrier, or completely drains out of the domain in the case of a constriction. This flow feature is analogous to the oscillations in lock-release gravity currents between the rear of the lock and the front of the flow (see Rottman & Simpson (1983) and Hogg (2006)). For flows through a constriction, the motion approaches a self-similar regime, the solution of which we construct (*cf.* Momen *et al.* (2017)), and the period of the oscillations grow exponentially. In contrast, for flows over a barrier the oscillations retain the period throughout, which is dependent upon the barrier height; this latter regime is analysed asymptotically as described below.

To reveal the behaviour at late times we develop a variation on multi-scale analysis (Bender & Orszag 1999; Kevorkian & Cole 1996). This asymptotic technique permits investigation of not only the systematic drainage of the flow over the barrier, but also the comparatively rapid dynamics of superimposed waves that propagate from one end of the lock to the other. The asymptotic analysis is carried out by introducing different time-scales for the amplitude and phase speed of waves and yields analytical expressions for the velocity and the depth of the fluid layer. The results are verified by comparison with the direct numerical integration of the governing shallow water equations and importantly draw out the wave structures that develop at relatively late times.

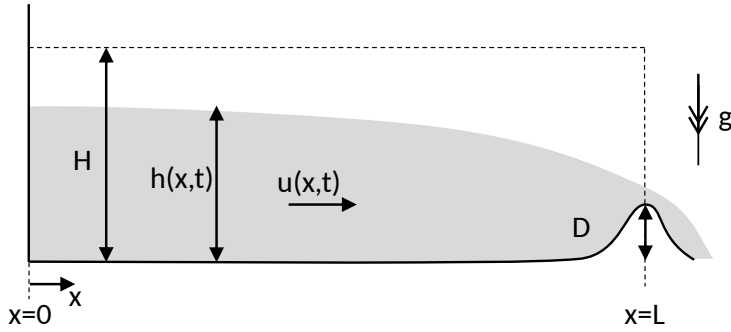


Figure 1: The configuration of the flow over a weir, showing the important variables. The vertical scale is exaggerated and the horizontal axis nonlinearly scaled to show the reservoir and the weir.

The numerical results are produced utilising a central scheme (Kurganov *et al.* 2001), which is designed to solve hyperbolic conservation laws with very low numerical dissipation, and resolve wave structures accurately over tens if not hundreds of periods. We implement boundary conditions using ghost cells, and there are several approaches of how to construct their values. Often extrapolation or reflection of the bulk values are used (Leveque 2002). Here a new approach is employed, where the ghost cell values are evolved in a manner compatible with both the characteristic structure of the bulk equation and the boundary conditions. This produces a system of differential algebraic equations (Kunkel & Mehrmann 2006; Ascher & Petzold 1998), for which we provide a new numerical method consistent with the time-stepping (Shu & Osher 1988). As will be shown (§3 and §4), the numerical method is capable of producing results that are in very close agreement with those generated from hodograph techniques described above.

The paper is structured as follows. First we formulate the problem in terms of the shallow water equations and derive the boundary conditions (§2 and Appendix A). The equations form a hyperbolic system, which are studied using the quasi-analytical techniques set out in §2.1. Also we integrate the nonlinear shallow water equations directly using the method described in §2.2. The analytical, numerical and asymptotic techniques are then applied to problems of unsteady fluid draining over a (possibly constricted) barrier (§3) and through a pure constriction (§4). In §3.1, we derive the initial phases before the motion is affected by the end wall of the lock using the method of characteristics and then in §3.2 this is extended using hodograph techniques to reveal how the confinement influences the unsteady drainage and leads to wave propagation between the lock and the barrier. Results from numerical integration of the shallow water equations extend these analytical results to later times, in which regime the motion can be tackled using asymptotic analysis (§3.3). The asymptotic results reveal the progressive draining and relatively rapid wave propagation; the algebraic details that are required to establish the temporal dependence of the amplitude and phase speed of the waves are presented in Appendix B, but comparison with the numerical results is presented in §3.3. Flows through constrictions are analysed similarly, using quasi-analytical and numerical techniques §4. In this latter scenario the motion unsteadily drains and develops relatively fast moving waves, the period of which grows exponentially in time. At late times, the motion is given by a similarity solution (§4.2), and we show that by computing the linear stability of this solution, we may deduce the exponential growth of the period of the oscillations (§4.3). We summarise and conclude the study in §5.

2. Formulation

In this contribution we model the unsteady, free-draining of a horizontal reservoir of fluid over a confining barrier or through a constriction. The fluid in the reservoir is initially quiescent and its gravitationally-driven motion arises following the partial removal of one of the end walls, allowing fluid to drain unsteadily (see figure 1 for a schematic of the configuration of the flow and important variables). In this investigation we neglect flow-induced mixing between the fluid initially in the reservoir and the surrounding fluid and thus our modelling framework applies to scenarios in which the fluids are effectively immiscible. We assume that the barrier and/or the constriction are localised to one end of the reservoir, so that away from this draining boundary the reservoir is of constant width and its base is at a constant elevation. The motion is also assumed to be shallow, so that the depth of the flowing layer is much smaller than the streamwise lengthscale, which implies that the velocity is predominantly parallel with the underlying boundary, vertical fluid accelerations are negligible and the pressure adopts a hydrostatic distribution. Thus in the bulk of the reservoir, away from the outflow boundary, the motion is unidirectional, and if viscous effects are negligible, dependent only on the streamwise coordinate and time; we may therefore adopt the shallow water model for the fluid motion (Peregrine 1972).

The depth of the flowing layer is denoted by $h(x, t)$ and the maximum height of the barrier by D ; both are rendered dimensionless with respect to the initial depth of the reservoir, H . The reservoir is of constant width, W , apart from within the locality of the boundary across which the fluid drains; the minimum width of the channel relative to the uniform width is denoted by w , and both the minimum width and maximum height occur at the same location. The horizontal flow velocity is denoted by $u(x, t)$ and it is made dimensionless with respect to $(\Delta\rho g H / \rho)^{1/2}$, where $\Delta\rho$ is the density difference between the reservoir and ambient fluids and ρ is the density of the reservoir fluid. Both dependent variables are functions of x and t , the distance from the back wall of the reservoir and the time since initiation, respectively. The x -axis is directed streamwise and is non-dimensionalised with respect to the reservoir length, L . Finally time is made dimensionless with respect to $L/(\Delta\rho g H / \rho)^{1/2}$. The dimensionless governing equations, which express conservation of mass and the balance of streamwise momentum, are then given by

$$\frac{\partial h}{\partial t} + \frac{\partial}{\partial x}(uh) = 0, \quad (2.1a)$$

$$\frac{\partial}{\partial t}(uh) + \frac{\partial}{\partial x}\left(u^2h + \frac{1}{2}h^2\right) = 0. \quad (2.1b)$$

The backwall of the reservoir is impermeable to fluid, which implies the boundary condition

$$u(0, t) = 0 \quad (2.2)$$

Fluid freely drains over the other boundary at which we enforce a mass- and energy-conserving condition that permits the relatively tranquil flow from the reservoir to be connected to the more rapid draining flow (Dake 1983, §4.3.4), given by

$$u^2 + 2(h - D) - 3\left(\frac{uh}{w}\right)^{2/3} = 0, \quad \text{at} \quad x = 1. \quad (2.3)$$

Details of the derivation of this boundary condition are given in Appendix A. It is notable that by adopting the dimensionless velocity and timescales described above, neither the governing equations (2.1a)-(2.1b) nor the boundary conditions (2.2) and (2.3) feature the

densities of the fluids. We further note that it is convenient to write (2.3) in terms of the Froude number at the boundary, $Fr = u(1, t)/\sqrt{h(1, t)}$, in which case we find that

$$Fr^2 - 3 \left(\frac{Fr}{w} \right)^{2/3} = -2 \left(1 - \frac{D}{h} \right). \quad (2.4)$$

Physically relevant solutions corresponds to $0 \leq Fr \leq 1$ and we note that $Fr = 1$ for $D = 0$ and $w = 1$, recovering the standard condition for free-draining flow when in the absence of a barrier or constriction. This system of equations has been recently employed by Momen *et al.* (2017) to study unsteady gravity current drainage over an edge ($D = 0, w = 1$). We emphasise that the definition of this internal Froude number, Fr , at the fixed position, $x = 1$ is different from the Froude number introduced by Benjamin (1968), and more recently revisited by Ungarish & Hogg (2018), which expresses the conditions at the front of a gravity current.

2.1. Hodograph variables

The shallow water equations (2.1a) and (2.1b) are hyperbolic and may be written in terms of quantities $\alpha = u + 2h^{1/2}$ and $\beta = u - 2h^{1/2}$, which are invariant along characteristics (Whitham 1974). Thus we find that

$$\frac{d\alpha}{dt} = 0 \quad \text{on} \quad \frac{dx}{dt} = u + h^{1/2} \quad \text{and} \quad \frac{d\beta}{dt} = 0 \quad \text{on} \quad \frac{dx}{dt} = u - h^{1/2}. \quad (2.5)$$

It is also possible to interchange the dependent and independent variables to treat x and t as functions of the characteristic invariants, α and β . This hodograph transformation converts the non-linear governing equations into linear equations. In particular as shown by Hogg (2006), the characteristic equations (2.5) are now given by

$$\frac{\partial x}{\partial \beta} = \frac{(3\alpha + \beta)}{4} \frac{\partial t}{\partial \beta} \quad \text{on} \quad \alpha = \text{constant}, \quad (2.6a)$$

$$\frac{\partial x}{\partial \alpha} = \frac{(\alpha + 3\beta)}{4} \frac{\partial t}{\partial \alpha} \quad \text{on} \quad \beta = \text{constant}. \quad (2.6b)$$

This transformation is possible and invertible provided the Jacobian $J = (\alpha - \beta)(\partial t/\partial \alpha)(\partial t/\partial \beta)$ is finite and non-vanishing.

It is possible to find a fundamental solution to the adjoint governing equations and this Riemann solution can then aide the construction of the solution from conditions along the boundaries in the hodograph plane. This technique was used by Hogg and coworkers to solve some gravity current and hydraulic problems using quasi-analytical techniques (Antuono *et al.* 2009; Antuono & Hogg 2009; Goater & Hogg 2011; Hogg *et al.* 2011). These analyses deploy the Riemann function, which is given by Garabedian (1986)

$$B(a, b; \alpha, \beta) = \frac{(a - b)^3}{(a - \beta)^{3/2}(\alpha - b)^{3/2}} \mathcal{F} \left[\frac{3}{2}, \frac{3}{2}; 1; \frac{(a - \alpha)(\beta - b)}{(a - \beta)(\alpha - b)} \right], \quad (2.7)$$

where \mathcal{F} denotes a hypergeometric function. Integration around a closed curve, $\partial \mathcal{D}$, in the hodograph plane yields

$$\int_{\partial \mathcal{D}} \mathbf{f} \cdot d\mathbf{x} = 0, \quad (2.8)$$

where $\mathbf{f}(a, b; \alpha, \beta) = -V\hat{\mathbf{a}} + U\hat{\mathbf{b}}$, $d\mathbf{x} = da\hat{\mathbf{a}} + db\hat{\mathbf{b}}$ and

$$U = -\frac{3}{2(a-b)}tB + \frac{B}{2}\frac{\partial t}{\partial b} - \frac{t}{2}\frac{\partial B}{\partial b}, \quad (2.9)$$

$$V = \frac{3}{2(a-b)}tB + \frac{B}{2}\frac{\partial t}{\partial a} - \frac{t}{2}\frac{\partial B}{\partial a}. \quad (2.10)$$

Thus a and b are dummy variables for the evaluation of integral (2.8) around the closed path $\partial\mathcal{D}$ in the hodograph plane.

The method of characteristics and the hodograph transformation will be used to construct the solution for this freely-draining flow and will be shown to bring considerable insight to the ensuing dynamics, elucidating the nonlinear wave reflections from the backwall ($x = 0$) and the barrier or constriction ($x = 1$). Throughout this paper we will use the following terminology: complex wave region refers to a domain within which both characteristic variables are varying; simple wave region refers to a domain within which one of the characteristic variables is constant and the other varying; and uniform region refers to a domain within which both characteristic variables are constant.

2.2. Numerical Method

We also integrate the governing equations (2.1a) and (2.1b) numerically. This is done utilising a central-upwind scheme (Kurganov *et al.* 2001) with a minmod piecewise linear reconstruction (see Kurganov *et al.* (2001) equation (3.16) with $\theta = 1$). The resultant semi-discrete system is then time-stepped using a third order Runge-Kutta scheme composed of multiple Euler time-steps (Shu & Osher 1988). This scheme is positivity preserving, i.e. we always have $h \geq 0$, due to the CFL condition and lack of source terms.

To impose boundary conditions we use three ghost cells at each domain end, for which we must find values consistent with both the governing equations and the boundary conditions. We discuss below the process of finding values at the domain end itself, but to impose values in the ghost cells some extrapolation utilising the domain endpoint values and interior values is required. Zeroth order extrapolation is inconsistent with the first order reconstruction in the cells, and first order extrapolation can produce negative depths. To resolve this issue we consider the ghost cells to have size $\epsilon\Delta x$ (where Δx is the cell size in the interior) and limit $\epsilon \rightarrow 0$. Under this limit all consistent extrapolation procedures converge to enforce the values of h and uh at the domain end across all ghost cells.

To deduce how the domain endpoint values should be evolved we consider the general form of a homogeneous system of m hyperbolic PDEs of a variable $Q(x, t) \in \mathbb{R}^m$

$$\frac{\partial Q}{\partial t} + A(Q, x, t)\frac{\partial Q}{\partial x} = \Psi(Q, x, t). \quad (2.11)$$

For the purposes of imposing boundary conditions alone, (2.11) is adjusted by including a small amount of diffusion $0 < \mathcal{D} \ll 1$, and then multiplying by the p^{th} left eigenvector of $A(Q, x, t)$, $l^p(Q, x, t)$ corresponding to eigenvalue $\lambda^p(Q, x, t)$, which results in the characteristic form

$$l^p \frac{\partial Q}{\partial t} + \lambda^p l^p \frac{\partial Q}{\partial x} = \mathcal{D} l^p \frac{\partial^2 Q}{\partial x^2} + l^p \Psi. \quad (2.12)$$

Diffusion is required for cases when a weak solution to the system contains a discontinuity between the bulk and the boundary, that is when the value at the boundary is not the limit of the interior values. Such a situation is not permitted in the vanishing viscosity solution, and to ensure this solution is found we include a small amount of diffusion. The boundary conditions are of the form $f(Q, t) = 0$, where the function $f : \mathbb{R}^m \times \mathbb{R} \rightarrow \mathbb{R}^k$,

and specify the values advected by characteristics entering the domain. We focus our discussion on the right hand end of the domain, that is we are at $x = x_e$ such that the domain is $x < x_e$ (and in this study $x_e = 1$). This means that we must enforce (2.12) for those p such that $\lambda^p \geq 0$ as well as the boundary conditions to determine $Q_e(t) = Q(x_e, t)$. We discretize the x derivatives using finite differences over the grid with cell size, Δx , and take $\mathcal{D} = \max_p |\lambda^p| \Delta x$ which is of the same form as the effective diffusion from the method in the bulk and is stable under the same CFL condition. This yields a system of differential algebraic equations (DAE) (see Kunkel & Mehrmann (2006) and Ascher & Petzold (1998)) of the form

$$L(Q_e, t) \frac{dQ_e}{dt} = C(Q_e, t), \quad (2.13a)$$

$$f(Q_e, t) = 0 \quad (2.13b)$$

where the rows of L are the $m - k$ left eigenvectors with $\lambda^p \geq 0$, and we treat the values of the internal cells as known functions of t so that their values do not appear in the arguments of C . For the shallow water equations and boundary conditions given by (2.1a), (2.1b), (2.2) & (2.3), we note that $m = 2$ and $k = 1$; thus at both boundaries, the rear wall ($x = 0$) and the barrier ($x = 1$), there is one outward propagating characteristic and one boundary condition to be applied. We denote the discrete times by t^n , the values at these discrete times $Q_e^n = Q_e(t^n)$, and the difference over a time-step of size Δt by $\Delta Q_e^n = Q_e^{n+1} - Q_e^n$. To be consistent with the time-stepping of the ODE portion of the DAE (2.13a) is discretized using Euler's method, and for the algebraic portion (2.13b) we enforce $f = 0$ at time t^{n+1} using a single step of the Newton-Raphson method with initial guess of the difference $\Delta Q_e^{n,0}$. This results in the explicit scheme

$$\begin{bmatrix} L(Q_e^n, t^n) \\ F(Q_e^n + \Delta Q_e^{n,0}, t^{n+1}) \end{bmatrix} \Delta Q_e^n = \begin{bmatrix} C(Q_e^n, t^n) \Delta t \\ F(Q_e^n + \Delta Q_e^{n,0}, t^{n+1}) \Delta Q_e^{n,0} - f(Q_e^n + \Delta Q_e^{n,0}, t^{n+1}) \end{bmatrix}, \quad (2.14)$$

where $F = \partial f / \partial Q$ and is non-vanishing, which is similar to the poststabilisation method with one iteration of projection (Ascher & Petzold 1998, §10.2).

As we are solving an ODE on a manifold (2.13) using Euler's method, we can show the the problem under consideration has sufficient smoothness conditions so that the method has a truncation error of $O(\Delta t)$. Similarly, if $\overline{\Delta Q_e^n}$ is the limit of iterating (2.14) with the output ΔQ_e^n being taken as the next input $\Delta Q_e^{n,0}$ (the implicit version of (2.14)), then we can show that the Newton-Raphson portion of our method will provide $f(Q^{n+1}, t^{n+1}) = O(\|\overline{\Delta Q_e^n} - \Delta Q_e^{n,0}\|^2)$ and then if $\|\overline{\Delta Q_e^n} - \Delta Q_e^{n,0}\| = O(\Delta t)$, we deduce that the convergence to the manifold is second order. Finally we can use the fact that, as time evolves, we are repeatedly performing Newton-Raphson steps on very similar problems to improve the scheme further. By taking our guess to be the value of a recent time step, for example $\Delta Q_e^{n,0} = \Delta Q_e^{n-1}$, then $f(Q^{n+1}, t^{n+1}) = O(\Delta t^4)$ and the scheme provides fourth order convergence to the manifold.

To use this method with the Runge-Kutta time steps from Shu & Osher (1988) some modifications must be made. These time steps are composed of multiple Euler time steps, which we shall denote by the operator E , and the intermediate results by $Q_e^{(i)}$ where $Q_e^{(0)} = Q_e^n$. The intermediate results are from computations of the form

$$Q_e^{(i+1)} = \eta_i Q_e^{(0)} + (1 - \eta_i) E(Q_e^{(i)}) \quad (2.15)$$

where $0 \leq \eta_i \leq 1$. Based on the results presented in Kunkel & Mehrmann (2006, §5.2) we argue that it is appropriate to approximately enforce $f(Q_e^{(i+1)}, t^{(i+1)}) = 0$, where $t^{(i)}$ is the time at which $Q_e^{(i)}$ is an approximation, rather than $f(E(Q_e^{(i)}), t^{(i)} + \Delta t) = 0$ as

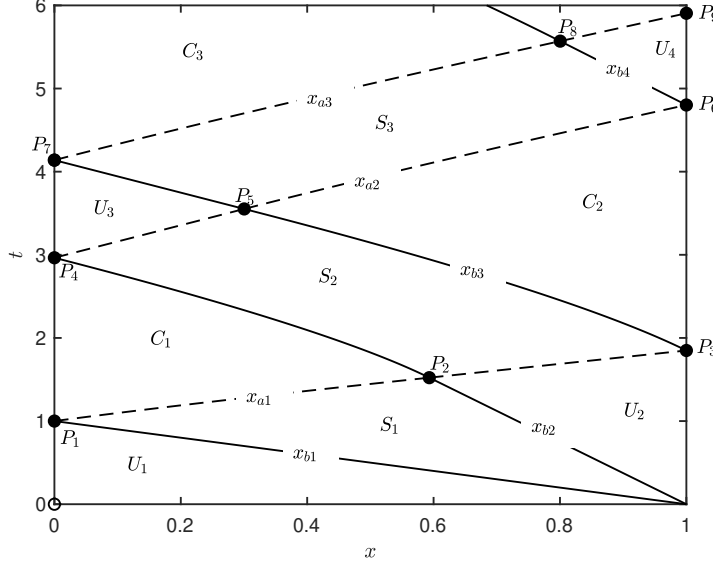


Figure 2: The characteristic plane for the unsteady draining of fluid from a partially confined reservoir with barrier height, $D = 0.05$ and width $w = 1$. In this figure we plot and label the uniform, simple and complex wave regions following the initiation of the flow. We also plot the characteristics bounding the wave regions; the suffices a and b denote the α - and β -characteristics and are plotted with dashed and solid lines, respectively. (These characteristic curves are defined in §3.) Additionally we label key points in the characteristic plane, which occur at the boundaries of the wave regions.

in (2.13). We therefore use a modified version of our scheme suitable for the application of (2.15), specifically

$$\begin{bmatrix} L(Q_e^{(i)}, t^{(i)}) \\ (1 - \eta_i)F(Q_e^{(i)*}, t^{(i+1)}) \end{bmatrix} \Delta Q_e^{(i)} = \begin{bmatrix} C(Q_e^{(i)}, t^{(i)})\Delta t \\ (1 - \eta_i)F(Q_e^{(i)*}, t^{(i+1)})\Delta Q_e^{(i),0} - f(Q_e^{(i)*}, t^{(i+1)}) \end{bmatrix} \quad (2.16a)$$

where

$$Q_e^{(i)*} = \eta_i Q_e^{(0)} + (1 - \eta_i)(Q_e^{(i)} + \Delta Q_e^{(i),0}), \quad (2.16b)$$

and $E(Q_e^{(i)}) = Q_e^{(i)} + \Delta Q_e^{(i)}$. The initial guesses are taken as $\Delta Q_e^{(i),0} = \Delta Q_e^{(0)}$, except for when $i = 0$ in which case $\Delta Q_e^{(0),0} = \Delta Q_e^{n-1}$.

The numerical scheme has been validated against a variety of known solutions, but for the purposes of this paper we simply observe the excellent correspondence between the simulations and analytic solutions at early times (see §3.2 and §3.3), and the convergence to the asymptotic wave structure at late times, for the particular problem considered here.

3. Draining over a weir

In this section we analyse the unsteady drainage of a reservoir from lock-release initial conditions over a weir of dimensionless height D and width w . We use the method of characteristics and hodograph techniques to construct the motion during the initial

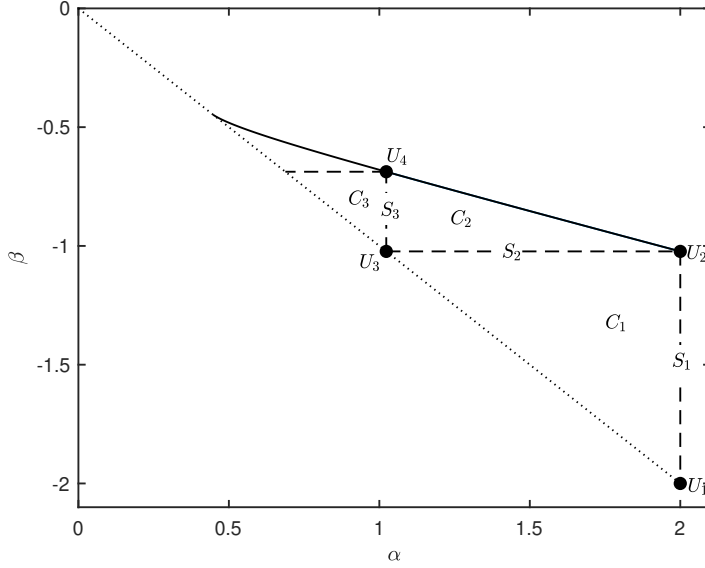


Figure 3: The hodograph plane for the unsteady draining of fluid from a partially confined reservoir, with barrier height, $D = 0.05$ and $w = 1$. In this figure, the solid line corresponds to the free-draining boundary at $x = 1$ and the dotted line to the impermeable boundary at $x = 0$. Uniform regions are depicted by points in the hodograph planes and are labelled U_i ($i = 1 - 4$), while simple wave regions correspond to dashed straight line segments aligned with the axes and are labelled S_i ($i = 1 - 3$). Finally, complex wave regions are labelled C_i ($i = 1 - 3$). This figure shows the wave pattern that emerges following flow initiation and the first three reflections from the end wall and freely-draining boundary. This pattern of successive reflections will continue indefinitely, approaching the end-state in which the fluid is quiescent and of depth equal to the barrier height; this is where the two boundary curves intersect at $\alpha = -\beta = 2\sqrt{D}$.

phases (§3.1, §3.2), while the motion at late times is found asymptotically (§3.3). We also present results from the direct numerical integration of the governing equations.

3.1. Initial motion: $t \leq 1$

The drainage condition (2.3) yields Froude numbers in the range $0 \leq Fr < 1$ (provided $D > 0$) and so at that boundary ($x = 1$) there is one out-flowing characteristic direction and one inflowing. This means that it is straightforward to construct the initial solution (valid for $t \leq 1$) since the characteristic variable α is determined by the initial conditions. Thus throughout the portion of the domain associated with the initial conditions, we find that $\alpha = 2$ and within this portion there is a rarefaction centered on $x = 1$, which we term simple wave region S_1 . Within S_1 , $-2 \leq \beta \leq \beta_*$, where β_* is a function of D and w and is the maximum value of β in this region, determined from (2.3) with $\alpha = 2$. (Since $0 \leq Fr < 1$, we can show that $-2 \leq \beta_* < -2/3$.) The solution for the velocity and height fields within S_1 are given by

$$u = \frac{2}{3} \left(1 + \frac{(x-1)}{t} \right) \quad \text{and} \quad h = \frac{1}{9} \left(2 - \frac{(x-1)}{t} \right)^2, \quad (3.1)$$

for $x_{b1} \equiv 1 - t < x < x_{b2} \equiv 1 + (2 + 3\beta_*)t/4$ (see figure 2). Upstream of this rarefaction, the fluid remains undisturbed from its initial state with $h = 1$ and $u = 0$; we denote this uniform region as U_1 . Downstream of the rarefaction, there is a uniform region attached to the right boundary ($x = 1$), within which $u = 1 + \beta_*/2$ and $h = (2 - \beta_*)^2/16$; we denote this as region U_2 . Example profile of height and velocity are plotted in figure 5. We observe that the fluid slumps gravitationally, progressively mobilising fluid within the reservoir and that its motion is ‘choked’ by draining at the outflow. These solutions for the flow are valid until the rearward propagating characteristic reaches the backwall of the reservoir, which occurs at $t = 1$. Thereafter there are reflections from this wall and the motion becomes more complicated.

3.2. Motion for $t \geq 1$

When $t \geq 1$, the structure of the solution for the height and velocity fields is altered due to reflection first from the back wall of the reservoir and then from the freely draining boundary at the front. It is useful to treat these dynamics in the characteristic plane (figure 2) and in the hodograph plane (figure 3), both of which reveal the successive wave reflections. As described above, the initial phase of the motion involves a simple wave region, S_1 , sandwiched between two uniform regions, U_1 and U_2 , which are attached to the boundaries of the reservoir. In the hodograph plane, a simple wave region corresponds to a line segment parallel to the coordinate axes and S_1 is given by $\alpha = 2$ and $-2 \leq \beta \leq \beta_*$. Uniform regions correspond to points in the hodograph plane; U_1 is given by $(\alpha, \beta) = (2, -2)$ and U_2 by $(\alpha, \beta) = (2, \beta_*)$. Complex wave regions correspond to connected domains in the hodograph plane (see figure 3).

The reflection from the back wall generates a complex wave region C_1 , which is bounded by the characteristic curve on which $\alpha = 2$. We denote that curve parametrically by $x_{a1}(\beta)$ and $t_{a1}(\beta)$ and it is straightforward to show that

$$t_{a1} = \frac{8}{(2 - \beta)^{3/2}} \quad \text{and} \quad x_{a1} = 1 + \frac{2(2 + 3\beta)}{(2 - \beta)^{3/2}}. \quad (3.2)$$

The region, C_1 , is also bounded by the characteristic on which $\beta = \beta_*$, denoted by $x_{b2}(t)$, which occurs at the leading edge of the rarefaction fan in the simple wave region, S_1 (see figure 2). It is given from (3.1) by $x_{b2} = 1 + (2 + 3\beta_*)t/4$. The curve x_{a1} intersects x_{b2} at the point P_2 with coordinates, $(x, t) = (1 + 2(2 + 3\beta_*)/(2 - \beta_*)^{3/2}, 8/(2 - \beta_*)^{3/2})$. Thereafter x_{a1} follows a straight line that bounds the uniform region, U_2 until reaching the outflow at $x = 1$. In this portion the characteristics are given by

$$\frac{x - x_{a1}(\beta_*)}{t - t_{a1}(\beta_*)} = \frac{6 + \beta_*}{4}. \quad (3.3)$$

and the point, P_3 , at the outflow is $(1, 16/[(6 + \beta_*)(2 - \beta_*)^{1/2}])$. The complex wave region, C_1 , may be calculated using the Riemann function, integrated around a trajectory in the hodograph plane, as demonstrated by Hogg (2006); it is compactly given by

$$t(\alpha, \beta) = B(2, -2; \alpha, \beta), \quad (3.4)$$

and the solution for x is given by integrating along α -characteristics from the back wall, using (2.6a) to show

$$x(\alpha, \beta) = \frac{(3\alpha + \beta)}{4}t(\alpha, \beta) - \frac{\alpha}{2}t(\alpha, -\alpha) - \frac{1}{4} \int_{-\alpha}^{\beta} t(\alpha, b) db. \quad (3.5)$$

In the characteristic plane, the continuation of the $\beta = \beta_*$ characteristic beyond P_2

bounds the region C_1 (see figure 2). It meets the back-wall at $P_4 = (0, t_{b2}(-\beta_*))$, where

$$t_{b2}(\alpha) = B(2, -2; \alpha, -\beta_*). \quad (3.6)$$

The β -characteristics starting from the uniform region, U_2 generate the simple wave region, S_2 , within which $-\beta_* \leq \alpha \leq 2$ (see figures 2 and 3). The trajectory of the characteristic from point P_3 on which $\beta = \beta_*$ is denoted by (x_{b3}, t_{b3}) and satisfies

$$\frac{\partial x_{b3}}{\partial \alpha} = \frac{(\alpha + 3\beta_*)}{4} \frac{\partial t_{b3}}{\partial \alpha}. \quad (3.7)$$

However on α -characteristics within S_2 , we also find that

$$\frac{x - x_{b2}}{t - t_{b2}} = \frac{3\alpha + \beta_*}{4}. \quad (3.8)$$

Thus we may establish

$$t_{b3} - t_{b2} = \frac{-8(2 + 3\beta_*)}{(6 + \beta_*)(\alpha - \beta_*)^{3/2}} \quad (3.9)$$

and then $x_{b3} - x_{b2}$ may be deduced from (3.8). This β -characteristic, (x_{b3}, t_{b3}) , forms one of the boundaries to the complex wave region at the front, C_2 , and to the uniform region, U_3 , adjacent to the back wall. In particular the point P_5 is determined by the first location along this characteristic on which the velocity vanishes, which is given by $\alpha = -\beta = -\beta_*$. Beyond P_5 , the β -characteristic is a straight line given by

$$\frac{x - x_{b3}(-\beta_*)}{t - t_{b3}(-\beta_*)} = -\frac{\beta_*}{2}. \quad (3.10)$$

This straight segment intersects the back wall at point $P_7 = (0, t_{b3}(-\beta_*) + 2x_{b3}(-\beta_*)/\beta_*)$.

Constructing the solution within the complex wave region, C_2 entails the use of the Riemann function (2.7) and the conversion of the boundary conditions into hodograph variables. In particular the free-draining condition, given by (2.3), may be written as

$$\frac{dx}{d\alpha} = 0 \quad \text{on} \quad \beta = \beta_s(\alpha), \quad (3.11)$$

We note that given a barrier height, D , that $\beta_s(\alpha)$ determines a curve in the hodograph plane such that $\beta_s(2) = \beta_*$ and that $\beta(2\sqrt{D}) = -2\sqrt{D}$, the latter condition corresponding to no flow when the fluid depth matches the height of the barrier. (This function, $\beta_s(\alpha)$, is plotted as a solid curve in figure 3 for $D = 0.05$.) From (2.3), we also find that

$$\frac{d\beta_s}{d\alpha} = \frac{(Fr^{4/3} - 1/w^{2/3})(Fr - 2) - 2DFr^{1/3}/h}{(Fr^{4/3} - 1/w^{2/3})(Fr + 2) - 2DFr^{1/3}/h}, \quad (3.12)$$

and this implies that $d\beta_s/d\alpha = -1$ when $Fr = 0$. Thus the curve β_s approaches its end point at $\beta_s = -2\sqrt{D}$ tangentially to the line $\beta = -\alpha$ (see figure 3). (The case without a barrier is different: when $D = 0$, the outflow Froude number, Fr , does not vary with the flow depth, h and so the boundary curve, β_s , is a straight line of gradient $d\beta_s/d\alpha = (Fr - 2)/(Fr + 2)$.) Using the characteristic equations (2.6a) and (2.6b), we can show that the boundary condition (3.11) is given by

$$2(\alpha + \beta_s) \left(\frac{\partial t}{\partial \alpha} + \frac{d\beta_s}{d\alpha} \frac{\partial t}{\partial \beta} \right) + (\beta_s - \alpha) \left(\frac{\partial t}{\partial \alpha} - \frac{d\beta_s}{d\alpha} \frac{\partial t}{\partial \beta} \right) = 0 \quad \text{on} \quad \beta = \beta_s(\alpha), \quad (3.13)$$

The other perimeter data in the hodograph plane are supplied by $t = t_{b3}(\alpha)$ along $\beta = \beta_*$. We then integrate around the closed curve $OQRT$, as depicted in figure 4. The coordinates of these points in the hodograph plane are O , $(2, \beta_*)$; Q , (α, β_*) ; R , (α, β_s) ;

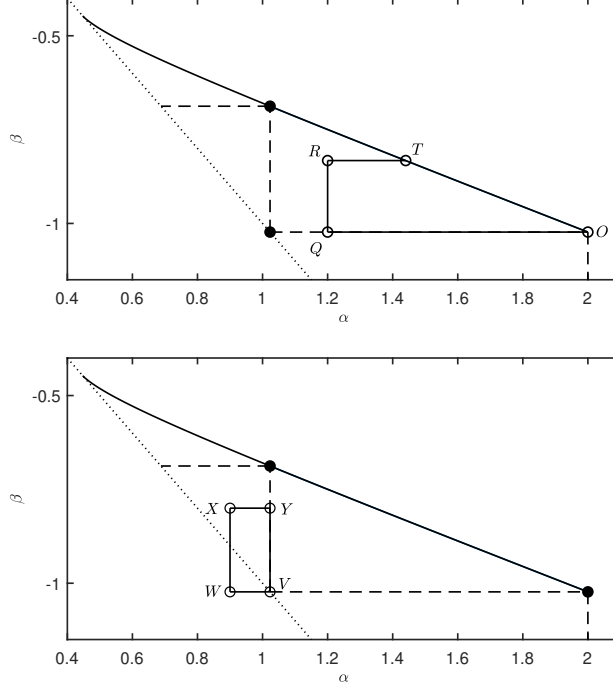


Figure 4: Sections of the hodograph plane showing: (a) the closed curve $OQRT$, which is used to construct the solution in the complex wave region, C_2 ; and (b) the rectangle $VWXY$, which is used to construct the solution in C_3 .

and T , (α_s, β_s) . The curves OQ , QR and RT correspond to straight line segments in the hodograph plane aligned with one of the coordinate axes, and the curve RT corresponds to the free-draining boundary condition ($\alpha = \alpha_s$ and $\beta = \beta_s$). Integration of (2.8) around $OQRT$ yields

$$\begin{aligned}
 t(\alpha, \beta_s) = & B(\alpha, \beta_s; \alpha, \beta_s) t_{b3}(\alpha) + B(2, \beta_s; \alpha, \beta_s) t_{b3}(2) \left(\frac{2 + \beta_s}{2 - \beta_s} - \frac{1}{2} \right) \\
 & + B(\alpha_s, \beta_s; \alpha, \beta_s) t(\alpha_s, \beta_s) \left(\frac{1}{2} - \frac{(\alpha_s + \beta_s)}{\alpha_s - \beta_s} \right) - \int_2^\alpha \left(\frac{\partial B}{\partial a} - \frac{3B}{2(a - \beta_s)} \right) t_{b3}(a) da \\
 & - \int_{\alpha_s}^2 \frac{t(a_s, b_s)}{2(a_s - b_s)} \left((3a_s + b_s) \left(\frac{\partial B}{\partial a} - \frac{B}{a_s - b_s} \right) + (a_s + 3b_s) \frac{db_s}{da} \left(\frac{\partial B}{\partial b} + \frac{B}{a_s - b_s} \right) \right) da.
 \end{aligned} \tag{3.14}$$

In the first integral of (3.14), the Riemann function and its derivative are evaluated at $b = \beta_s$, whereas in the final integral of (3.14), the Riemann function and its derivatives are evaluated along the curve TO given parametrically by $a = a_s$ and $b = b_s$. Before we can use this expression to evaluate t at a general position within the complex wave region, C_2 , we must first evaluate the time field along the boundary curve TO ($t_s(\alpha) \equiv t(\alpha, \beta_s(\alpha))$). Setting $\alpha = \alpha_s$ in (3.14), we derive an integral equation for t_s of the form

$$t_s(\alpha) = f_1(\alpha) + \int_2^\alpha f_2(a, \alpha) t_s(a) da, \tag{3.15}$$

where f_1 and f_2 are functions determined from (3.14). This is a Volterra integral equation

of the second kind and is readily solved by iteration; we find numerical convergence to a relative tolerance of 10^{-12} within 10 iterates. Example solutions are plotted in figures 5 and 6.

Given $t_s(\alpha)$, the dependent variables can be evaluated immediately by numerical quadrature, using (3.14) for $t(\alpha, \beta)$ and integrating along an α -characteristic using (2.6a) to determine $x(\alpha, \beta)$, with condition $x = x_{b3}$. This enables the boundary to C_2 to be computed as the α -characteristic on which $\alpha = -\beta_*$, starting from P_5 . The α -characteristic reaches the free-draining boundary at $P_6 = (1, t_s(-\beta_*))$.

Finally we note that we also construct the characteristic on which $\alpha = -\beta_*$ emanating from the point P_7 at the back wall. This curve denoted parametrically by (x_{a3}, t_{a3}) is determined from the characteristic equation (2.6a), which here yields

$$\frac{\partial x_{a3}}{\partial \beta} = \frac{(-3\beta_* + \beta)}{4} \frac{\partial t_{a3}}{\partial \beta}. \quad (3.16)$$

Also in the simple wave region, S_3 , in which $\alpha = -\beta_*$, we have that

$$\frac{x - x_{a2}}{t - t_{a2}} = \frac{-\beta_* + 3\beta}{4}. \quad (3.17)$$

Hence we deduce that

$$t_{a3} - t_{a2} = -\frac{2}{\beta_*} x_{b3}(-\beta_*) \left(\frac{-2\beta_*}{-\beta_* - \beta} \right)^{3/2}, \quad (3.18)$$

and then x_{a3} follows from (3.17). The uniform region adjacent to the freely-draining boundary corresponds to $(\alpha, \beta) = (-\beta_*, \beta_m)$, where $\beta_m = \beta_s(-\beta_*)$. Then we may determine P_8 as $(x_{a3}(\beta_m), t_{a3}(\beta_m))$ and P_9 as $(1, t_{a3}(\beta_m) + 4(1 - x_{a3}(\beta_m))/(\beta_m - 3\beta_*))$.

The construction of the solution may be extended to the complex wave region C_3 using the boundary data along the α -characteristic on which $\alpha = -\beta_*$; this data is $x_{a3}(\beta)$ and $t = t_{a3}(\beta)$, and is derived above. In addition we enforce the impermeability of the back wall (2.2) by imposing symmetry on the t -variable, so that $t(\alpha, \beta) = t(-\beta, -\alpha)$. We then integrate around a rectangle in the hodograph plane, $VWXY$, with edges aligned with the coordinate axes (see figure 4). The vertices of the rectangle are at $V = (-\beta_*, \beta_*)$, $W = (\alpha, \beta_*)$, $X = (\alpha, \beta)$ and $Y = (-\beta_*, \beta)$. The characteristic data, $t = t_{a3}(\beta)$ provides the solution on the boundary segment YV , while the imposed symmetry determines the solution on the boundary segment VW , $t(\alpha, \beta_*) = t(-\beta_*, -\alpha) = t_{a3}(-\alpha)$. We may then integrate (2.8) around $VWXY$ to yield

$$t(\alpha, \beta) = B(\alpha, \beta_*; \alpha, \beta) t_{a3}(-\alpha) + B(-\beta_*, \beta; \alpha, \beta) t_{a3}(\beta) - B(-\beta_*, \beta_*; \alpha, \beta) t_{a3}(\beta_*) - I_1 - I_2, \quad (3.19)$$

where the integrals I_1 and I_2 are given by

$$I_1 = \int_{-\beta_*}^{\alpha} \left(-\frac{3B}{2(a - \beta_*)} + \frac{\partial B}{\partial a} \right) t_{a3}(-a) da, \quad (3.20)$$

$$I_2 = \int_{\beta}^{\beta_*} \left(-\frac{3B}{2(-\beta_* - b)} - \frac{\partial B}{\partial b} \right) t_{a3}(b) db. \quad (3.21)$$

In the integrand of I_1 , $b = \beta_*$, whereas in the integrand of I_2 , $a = -\beta_*$. This construction is essentially the same as was used by Hogg (2006) to derive (3.4). The evaluation of $x(\alpha, \beta)$ follows immediately by integrating along α -characteristics (2.6a) and using the boundary condition that $x(\alpha, -\alpha) = 0$.

The description of the solution presented in the previous paragraphs completes the computation of the flow fields for the first three reflections; subsequent reflections could

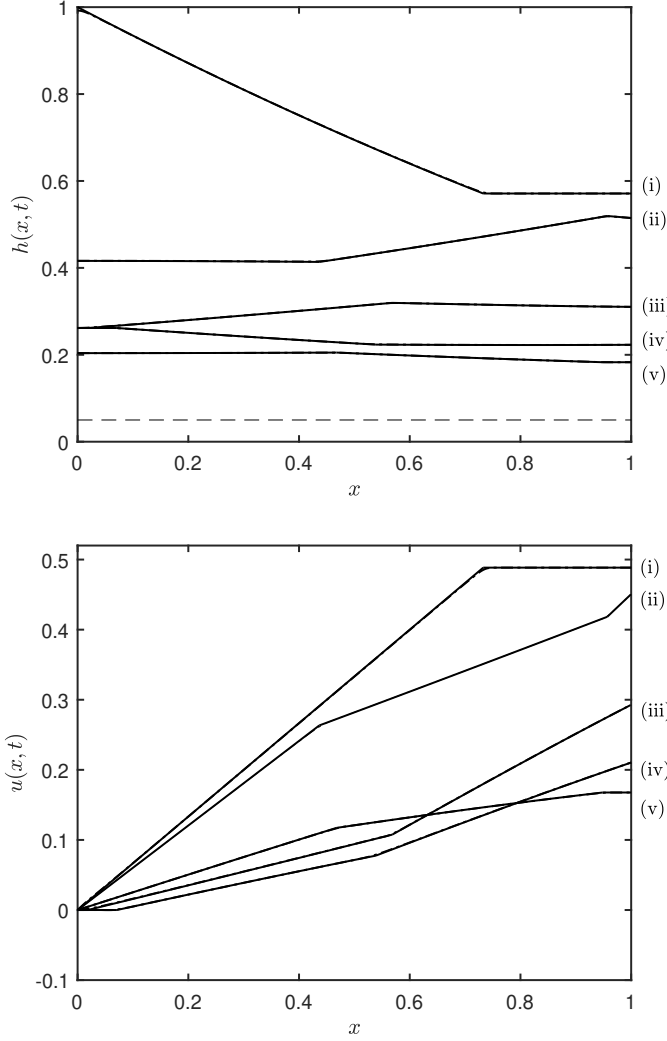


Figure 5: The height, $h(x, t)$ and velocity, $u(x, t)$, as functions of position at various instances of time, $t = 1, 2, 3, 4$ & 5 (labelled (i)-(v), respectively) for $D = 0.05$ and $w = 1$. The solid line depicts the solution calculated using the quasi-analytical hodograph methods, while the dot-dashed line depicts the solution from the direct numerical integration of the shallow water equations (2.1a)-(2.1b). We note that there is negligible difference between the curves, which overlie each other; the largest differences occur close to positions in the profiles where the solutions have discontinuous gradient. Such features are captured exactly by the analytical approach, whereas the numerical results are slightly smoothed. The dashed line indicates the height of the barrier ($D = 0.05$).

in principle be computed using an identical formulation to find the complex wave regions next to the freely-draining boundary and then to calculate the solutions close to the back wall. In the hodograph plane we can see that this sequence of regions forms an infinite ‘staircase’ towards the final state in which the fluid is quiescent of a flow depth equal to

the height of the barrier. Notably, each reflection will generate uniform regions adjacent to the boundaries.

We illustrate the solution by plotting the solutions for the height and velocity fields as function of position at various fixed times for $D = 0.05$ (see figure 5). The profiles, plotted at times $t = 1, 2, 3, 4$ and 5 , draw from the uniform, simple and complex wave regions illustrated in figure 2. We note that they feature the uniform regions adjacent to the boundaries and that they illustrate the wave reflections along the reservoir. For example, we see that the flow oscillates so that at times it is thickest close to the end-wall, while at other times it is thickest close to the freely-draining boundary. We also plot the height at the draining boundary, $h(1, t)$, as a function of time for various barrier heights and a constant width, $w = 1$, (figure 6a) and for various constrictions and a constant barrier height, $D = 0.1$ (figure 6b). We note that this height at the end of the channel has intervals within which it is constant, followed by periods within which it declines; this is in accord with the description that we have developed above. The successive wave reflections lead to uniform regions adjacent to the boundaries within which the height and velocity are constant; in figure 6, we observe the height variation within regions, U_1 , C_2 and U_4 (see figure 2) and we have calculated these using the techniques described above. In principle we could extend this further, but in this study we access later times numerically and through using a different analytical approach (§3.3).

3.3. Late times: numerical results and asymptotic analysis

From the structure of the hodograph solution we anticipate that at late times we will continue to see a ‘staircase’ of uniform and complex wave regions. This is consistent with our numerical simulations, computed up to time $t = 100$, and presented in figure 7. In this figure we plot the height excess over the barrier and the velocity incident on the barrier as functions of time for a selection of barrier heights. (We have introduced a time offset, t_0 and have also plotted the asymptotic trends, which are introduced and deduced below.) It is evident from these plots that the solution tends rapidly towards a power law, around which there are strong, but decaying, oscillations, whose period and shape are functions of the barrier height, D .

In what follows we investigate the asymptotic form of the solutions for the height and velocity fields trends at late times after the flow initiation. For this analysis we utilise a multi-scale asymptotic expansion in time to capture the progressive unsteady draining of the reservoir and the more rapid oscillations, the amplitude of which decay temporally and the phase speed of which approach a constant value dependent upon the barrier height. This analysis is based upon embedding a set of timescales into the dependent variables and then examining their evolution at relatively late times. We first construct these appropriate time-scales, then present the resulting expansion with the algebraic details confined to Appendix B. Finally we compare the asymptotic forms to the numerical simulations.

We investigate the solution at late times, where $t = O(1/\epsilon)$ and ϵ is a small, arbitrary ordering parameter ($0 < \epsilon \ll 1$). In the end, the solutions cannot depend upon ϵ , but its introduction is useful in organising the magnitude of various terms. We also introduce the following set of timescales, $\hat{T}_n(\epsilon, t)$ ($n = 0, 1, 2, \dots$), such that at the long timescale of interest $\hat{T}_n = O(\epsilon^{n-1})$. Provided that all variables other than t and \hat{T}_n are independent of ϵ we can always express the timescales as

$$\hat{T}_n(\epsilon, t) = \epsilon^{n-1} \omega_n(\epsilon t), \quad (3.22)$$

where ω_n are functions of order unity when $t = O(1/\epsilon)$. For the purposes of constructing an asymptotic expansion the timescale $\hat{T}_1 = \omega_1(\epsilon t)$ is equivalent to $\hat{T}_1 = \epsilon t$. Therefore,

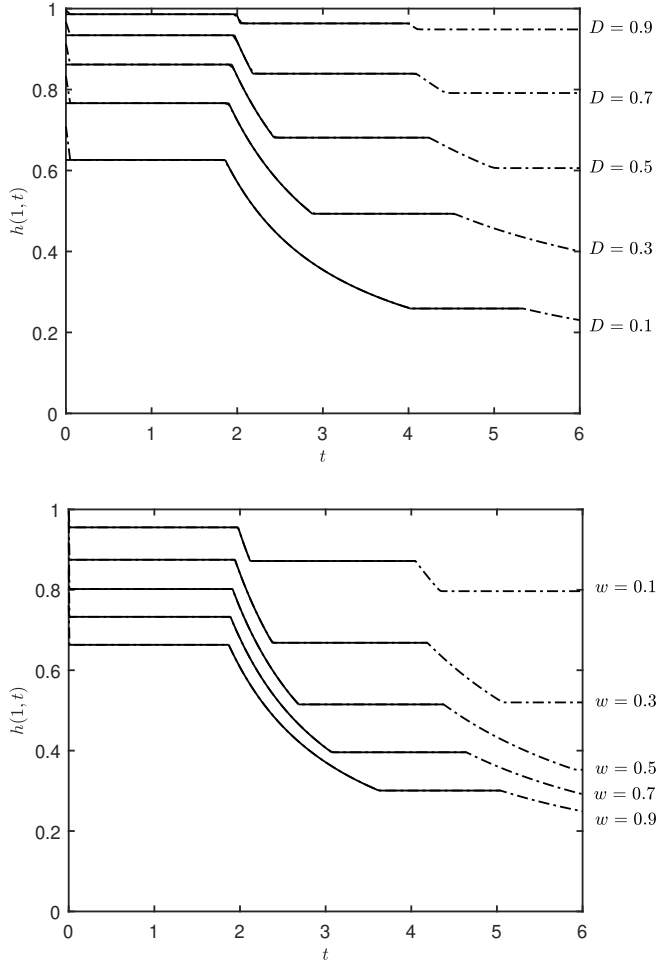


Figure 6: The height at the freely-draining boundary, $h(1, t)$, as a function of time for (a) various barrier heights, D , and $w = 1$; and (b) various constrictions, w , and $D = 0.1$. The solid lines correspond to results from integration in the hodograph plane and the dot-dashed lines correspond to numerical integration of the shallow water equations (2.1a)-(2.1b). The quasi-analytical hodograph calculations were performed until the end of region U_4 , but the numerical computations were continued over much longer times. We note the very close agreement between the two approaches, with the only very slight differences occurring at the junctions of the intervals during which the height is uniform or decreasing.

without loss of generality, we take

$$\hat{T}_0 = \epsilon^{-1}\omega_0(T_1), \quad \hat{T}_1 = \epsilon t, \quad \hat{T}_n = \epsilon^{n-1}\omega_n(T_1), \quad n \in \{2, 3, \dots\} \quad (3.23)$$

where $\omega'_n \neq 0$, $n \in \{0, 2, 3, \dots\}$.

For most problems multi-scale asymptotics fails to produce a closed system of equations for high order corrections involving high order time-scales, as mentioned in Bender & Orszag (1999, §11.2). To understand why this is, we consider a problem in m time-scales which, at each order, generates a new arbitrary functions dependent on $m - 1$ time-scales

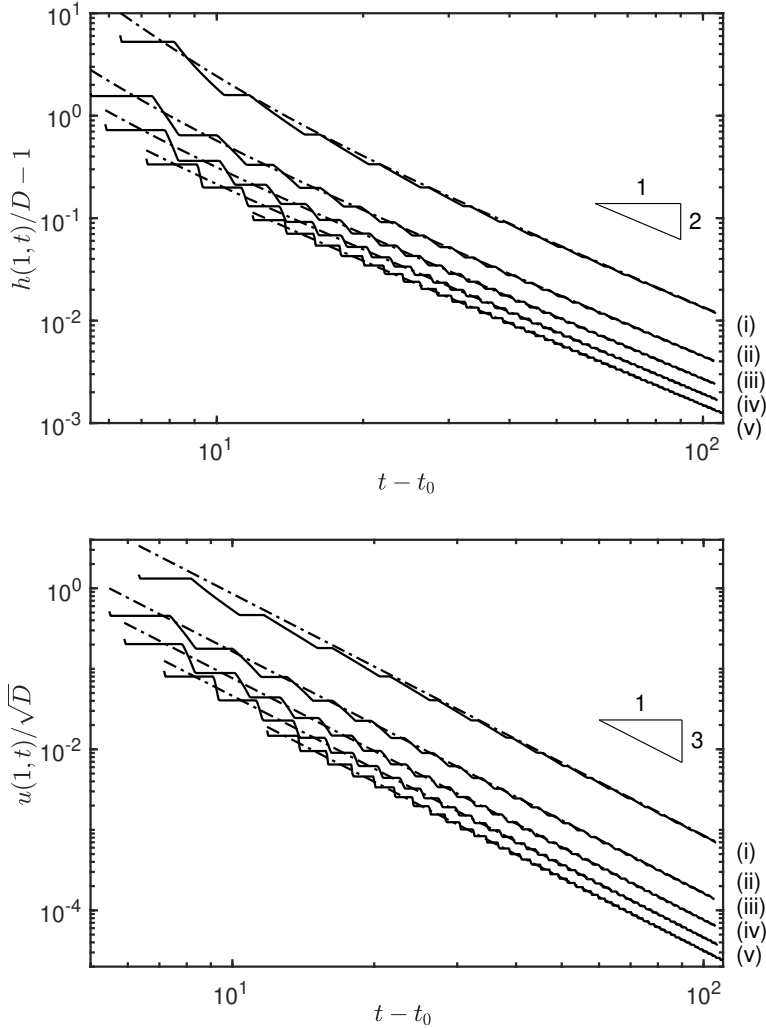


Figure 7: (a) The elevation of the fluid layer above the barrier relative to the height of the barrier, $h(1,t)/D - 1$; and (b) and the velocity at the outflow $u(1,t)/\sqrt{D}$ as functions of time for $w = 1$ and $D = 0.9, 0.7, 0.5, 0.3, 0.1$ (denoted (i)-(v), respectively). The solid lines are from numerical integration of the governing equations and reveal strong oscillatory behaviour superimposed upon progressive decay of each variable. The dashed-dot lines are the single scale asymptotic solution from (3.26a,b).

(the dependence on one of the time-scales being set at that order), along with s solvability conditions providing restrictions on the functions introduced at lower order. Considering an expansion including N terms, and ignoring contributions independent of N , this means there are a total of $a(m-1)N$ functional dependencies and sN solvability conditions, resulting in $[a(m-1) - s]N$ undetermined dependencies. To stress how pathologically bad this lack of closure is, we observe that if a full set of $O(N)$ time-scales is used then we will have $O(N^2)$ undetermined dependencies. If we are to have a closed expansion, introducing the same number of dependencies and conditions as we increase from order N to $N+1$, we must have $m = 1 + s/a$.

For many problems, such as the one considered here, $s = a$ and thus we require two time-scales. We anticipate that, as with many of the problems discussed in Kevorkian & Cole (1996), the time-scale \hat{T}_0 is the time-scale in which oscillations and waves occur, \hat{T}_1 governs the amplitude of these waves, and higher order time-scales act as a phase-shift to \hat{T}_0 in the form of a strained coordinate (Hinch 1992, §7.1). This results in the two-scale ansatz: that all time dependencies can be expressed by the time-scales

$$T_0 = k_0 \hat{T}_0 + k_2 \hat{T}_2 + k_3 \hat{T}_3 + \dots = \epsilon^{-1} \omega_0(T_1) + \epsilon \omega_2(T_1) + \epsilon^2 \omega_3(T_1) + \dots, \quad (3.24a)$$

$$T_1 = \hat{T}_1 = \epsilon t. \quad (3.24b)$$

where the coefficients for the linear combination k_n have been absorbed into ω_n .

Our asymptotic analysis then proceeds by assuming that the height and velocity fields are dependent on both timescales T_0 and T_1 as well as the spatial variable x . We substitute series expansion of the form (B 9b) into the governing equations (2.1a)-(2.1b) and deduce expressions that satisfy the boundary conditions and remain bounded as time evolves. These latter ‘secular’ conditions are key to determining the functions, ω_n , and the decaying amplitudes of the wave-like behaviour.

It is convenient to write the solutions for the dependent fields as the sum of two contributions,

$$h = h_a + h_w \quad \text{and} \quad u = u_a + u_w, \quad (3.25)$$

where h_a and u_a are asymptotic components of the solutions that depend on T_1 (and will be henceforth termed the ‘single time-scale asymptotics’) and h_w and u_w are the wave-like components. From Appendix B, we find that correct to $O(1/(\sqrt{D}(t-t_0))^5)$

$$\frac{h_a(x, t)}{D} = 1 + \frac{27}{2w^2(\sqrt{D}(t-t_0))^2} + \frac{27(3x^2+5)}{2w^2(\sqrt{D}(t-t_0))^4}, \quad (3.26a)$$

$$\frac{u_a(x, t)}{\sqrt{D}} = \frac{27x}{w^2(\sqrt{D}(t-t_0))^3}, \quad (3.26b)$$

$$\begin{aligned} \frac{h_w(x, t)}{D} &= \frac{1}{(\sqrt{D}(t-t_0))^3} [F(\eta) + F(\xi)] + \\ &\quad \frac{1}{(\sqrt{D}(t-t_0))^4} \left[\frac{w^2}{9} (F(\eta)^2 + F(\xi)^2) + 3x (F(\eta) - F(\xi)) \right], \end{aligned} \quad (3.26c)$$

$$\begin{aligned} \frac{u_w(x, t)}{\sqrt{D}} &= \frac{1}{(\sqrt{D}(t-t_0))^3} [F(\eta) - F(\xi)] + \\ &\quad \frac{1}{(\sqrt{D}(t-t_0))^4} \left[\frac{w^2}{9} (F(\eta)^2 - F(\xi)^2) + 3x (F(\eta) + F(\xi)) \right], \end{aligned} \quad (3.26d)$$

where

$$\xi(x, t) = \sqrt{D}t - \frac{27}{4w^2\sqrt{D}(t-t_0)} + x, \quad \text{and} \quad \eta(x, t) = \sqrt{D}t - \frac{27}{4w^2\sqrt{D}(t-t_0)} - x. \quad (3.27)$$

The variable t_0 denotes a temporal offset that we are free to choose. The arbitrary function F is related to the initial conditions that gave rise to the unsteady draining flow. In terms of the variables in the appendix, it is given by $F(y) = D\hat{F}_3(y - \sqrt{D}\phi)$, and is thus of period 2. The function is affected by the choice of t_0 via the transformation

$$t_0 \mapsto t_0 + k, \quad \text{leads to} \quad F \mapsto F + \frac{27k}{2\sqrt{D}w^2}. \quad (3.28)$$

Thus we may choose F to have zero integral over each period by choosing t_0 , which means that the wave component oscillates around the single scale component. From (3.26) we see that D only affects the scales of h , u and t , which is equivalent to choosing to make the variables dimensionless with respect to HD rather than H . The constant small parameter ϵ used to compute the solution at the time t must have magnitude approximately $1/t$ for ϵt to be order unity. We define the time dependant small parameter ϵ_t to mean the value of ϵ which rescales t to exactly unity, taking into account appropriate scales and offset, that is

$$\epsilon_t = \frac{1}{\sqrt{D}(t - t_0)}. \quad (3.29)$$

We compare these asymptotic results with the numerical simulations as follows: we suppose that we know t_0 as well as the dependent variables at some comparison time $t = t_c$, given by $h(x, t_c)$, $u(x, t_c)$, and we use these fields to determine the function F over a single period. The differences between the numerically computed fields and the single-scale asymptotic components, $h_{wn} = h(x, t_c) - h_a(x, t_c)$ and $u_{wn} = u(x, t_c) - u_a(x, t_c)$ are added and subtracted to give

$$\frac{h_{wn}(x, t_c)}{2D} + \frac{u_{wn}(x, t_c)}{2\sqrt{D}} = \frac{F(\eta_c)}{(\sqrt{D}(t_c - t_0))^3} + \frac{1}{(\sqrt{D}(t_c - t_0))^4} \left[\frac{w^2 F(\eta_c)^2}{9} + 3xF(\eta_c) \right], \quad (3.30a)$$

$$\frac{h_{wn}(x, t_c)}{2D} - \frac{u_{wn}(x, t_c)}{2\sqrt{D}} = \frac{F(\xi_c)}{(\sqrt{D}(t_c - t_0))^3} + \frac{1}{(\sqrt{D}(t_c - t_0))^4} \left[\frac{w^2 F(\xi_c)^2}{9} - 3xF(\xi_c) \right], \quad (3.30b)$$

where $\xi_c(x) = \xi(x, t_c)$, $\eta_c(x) = \eta(x, t_c)$. These expressions apply for $0 \leq x \leq 1$. We may write (3.30a) and (3.30b) in a compact way by introducing the period 2 function $S(y)$, defined by

$$S(y) = (\sqrt{D}(t_c - t_0))^3 \left[\frac{h_{wn}(|\chi|, t_c)}{2D} - \text{sgn}(\chi) \frac{u_{wn}(|\chi|, t_c)}{2\sqrt{D}} \right], \quad (3.31)$$

where $y = \sqrt{D}t_c - 27/(4w^2\sqrt{D}(t_c - t_0)) + \chi$ and $-1 < \chi < 1$. Therefore the function F satisfies the following quadratic equation,

$$F^2(y) - 2BF(y) - C = 0 \quad (3.32)$$

where

$$B = \frac{27\chi - 9\sqrt{D}(t_c - t_0)}{2w^2}, \quad \text{and} \quad C = \frac{9\sqrt{D}(t_c - t_0)}{w^2} S(y) \quad (3.33)$$

We expect that $F \sim S$ for $t_c \gg t_0$, which selects the solution to (3.32) given by

$$F = B + \sqrt{B^2 + C}. \quad (3.34)$$

Finally, we evaluate the temporal offset, t_0 , by imposing

$$\int_0^2 F(y) dy = 0, \quad (3.35)$$

In table 1 we present the evaluation of the temporal offset, t_0 , for various barrier heights, D , and uniform width, $w = 1$, using either the initial conditions ($t_c = 0$) or the numerically determined profiles at $t_c = 100$. We find that the two values of t_0 for each barrier height are broadly similar; the relative difference is less than 10% and systematically decreases with D . The computed value of ϵ_t given in table 1 according to

Barrier height D	Offset, t_0 ($t_c = 0$)	Offset, t_0 ($t_c = 100$)	ϵ_t ($t_c = 0$)	ϵ_t ($t_c = 100$)	Relative difference in t_0
0.1	-6.3243	-6.8821	0.500	0.030	8.11%
0.3	-5.4772	-5.7104	0.333	0.017	4.08%
0.5	-5.8963	-5.9839	0.240	0.013	1.46%
0.7	-7.1605	-7.1895	0.167	0.011	0.40%
0.9	-11.8471	-11.8498	0.089	0.009	0.02%

Table 1: The temporal offset, t_0 , for various values of the barrier height D with $w = 1$, computed using (3.35) enforced at time $t_c = 0$ and $t_c = 100$. (For the latter, the dependent variables, h and u , were evaluated from the numerical simulations.). The corresponding values of ϵ_t are also given, along with the relative difference in the two computed values of t_0 .

(3.29) provides an indication of the magnitude of the difference between the numerically computed profiles and their asymptotic forms. Indeed we find that even using the initial conditions ($t_c = 0$) to determine t_0 appears to provide an accurate means of determining the asymptotic form for the larger barrier heights ($D \gtrsim 0.5$).

In figure 7 we have plotted the single timescale asymptotics, h_a and u_a (see (3.26a)-(3.26b)) as functions of time at the end of the domain ($x = 1$). For these plots we evaluated the temporal offset using the initial data ($t_c = 0$). We observe that these asymptotic forms accurately capture the numerical computations over relatively long computational times, and thus at late times for all barrier heights the outflow velocity decays with the inverse cube of time, whereas the excess height decays as the inverse square of time. We further note that there are oscillations of diminishing amplitude about the progressive decay of the height and velocity fields, which in these figures correspond to periods during which the outflow velocity and height remain constant; these oscillations are captured in the wave-like behaviour, h_w and u_w and are discussed below.

At late times when the depth of the fluid at the outflow is close to the barrier height ($h - D \ll 1$), we find that the velocity field satisfies $u \sim w(h - D)^{3/2}/D$. Thus the flow speed is weakening and eventually viscous stresses, which are not included in the dynamical model, may become non-negligible. We show in Appendix A how to assess their magnitude within the shallow flow over the crest of the weir and for the typical parameter values at environmental scales, viscous effects appear negligible until $h - D \sim 10^{-3}$.

The rescaled volume flux at the outflow boundary, $u(1, t)h(1, t)/D^{3/2}$ is plotted in figure 8 as a function of rescaled time $\sqrt{D}(t - t_0)$ for various values of the barrier height, D . In this plot we have calculated the offset time, t_0 from the initial profile (i.e. $t_c = 0$), according to the procedure described above; values of t_0 are given as the second column of table 1. We note that for $\sqrt{D}(t - t_0) \gtrsim 4$, the evolution follows a universal behaviour around which each of the separate cases oscillate due to the successive wave reflections between the back wall and the outflow. Furthermore we have also plotted the analytically calculated asymptotic form, $u_a(1, t)h_a(1, t)/D^{3/2}$, where h_a and u_a are given by (3.26a) and (3.26b), respectively. We observe that this expression accurately matches the computed flux.

The shape of the waveform of the oscillations can also be determined asymptotically and may in some cases be deduced from the initial conditions. When $t_c = 0$, the fluid is stationary ($u(x, t_c) = 0$) and the reservoir is of unit depth ($h(x, t_c) = 1$). In this case

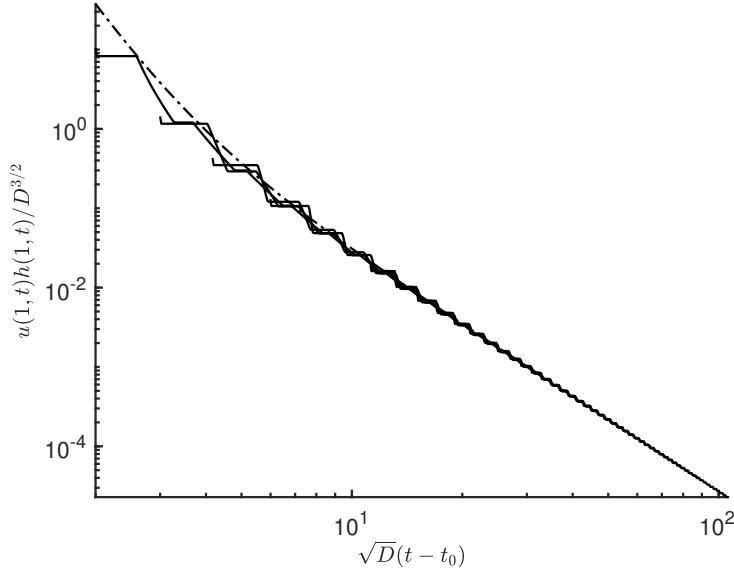


Figure 8: The rescaled volume flux of fluid leaving the domain at the outflow, $u(1, t)h(1, t)/D^{3/2}$, as a function of rescaled time $\sqrt{D}(t - t_0)$ with the temporal offset, t_0 , calculated from the initial profile, for $D = 0.1, 0.3, 0.5, 0.7$ and 0.9 (solid lines) and $w = 1$. The asymptotic form, $u_a(1, t)h_a(1, t)/D^{3/2}$, is plotted with a dot-dash line. In this figure, the curves from the different numerical computations overlay at late times and it is hard to distinguish them, indicating a universal behaviour that is accurately represented by the asymptotic form.

using the transform (3.28) and enforcing (3.35) we arrive at

$$F(y) = \frac{27\chi}{2w^2} \equiv \frac{27}{2w^2} \left(y - \sqrt{D}t_c + \frac{27}{4w^2\sqrt{D}(t_c - t_0)} \right) \quad (3.36)$$

for $-1 < \chi < 1$. This function, constructed to be periodic, contains a discontinuity at $\chi = \pm 1$. This is exactly the same as what would be produced if we had truncated our asymptotic expansion at a lower order and included terms up to $(t - t_0)^{-3}$. To understand this observation, we note that the rarefaction fan which smooths out the mismatch between the initial conditions and boundary condition at early times only exists because of the non-linearity of the shallow water equations. A linear system of hyperbolic partial differential equations would simply advect this mismatch as a discontinuity. Thus, we should only expect the asymptotic solution to capture this smoothing behaviour if we expand to sufficiently high order to capture the non-linear interaction of the waves, and to calculate (3.26) we only expanded to capture the non-linear effects of the boundary conditions and single-scale component, not the wave component. We therefore find that the asymptotic solution simply advects the initial mismatch as a discontinuity (which is what we see in figure 9). This failure to capture the non-linear self-interaction of the waves means we fail to capture the correct wave shape over the first few periods. We explore this effect by making comparisons between the simulation and asymptotic solution at a number of early times to find the time at which the non-linear effects become small

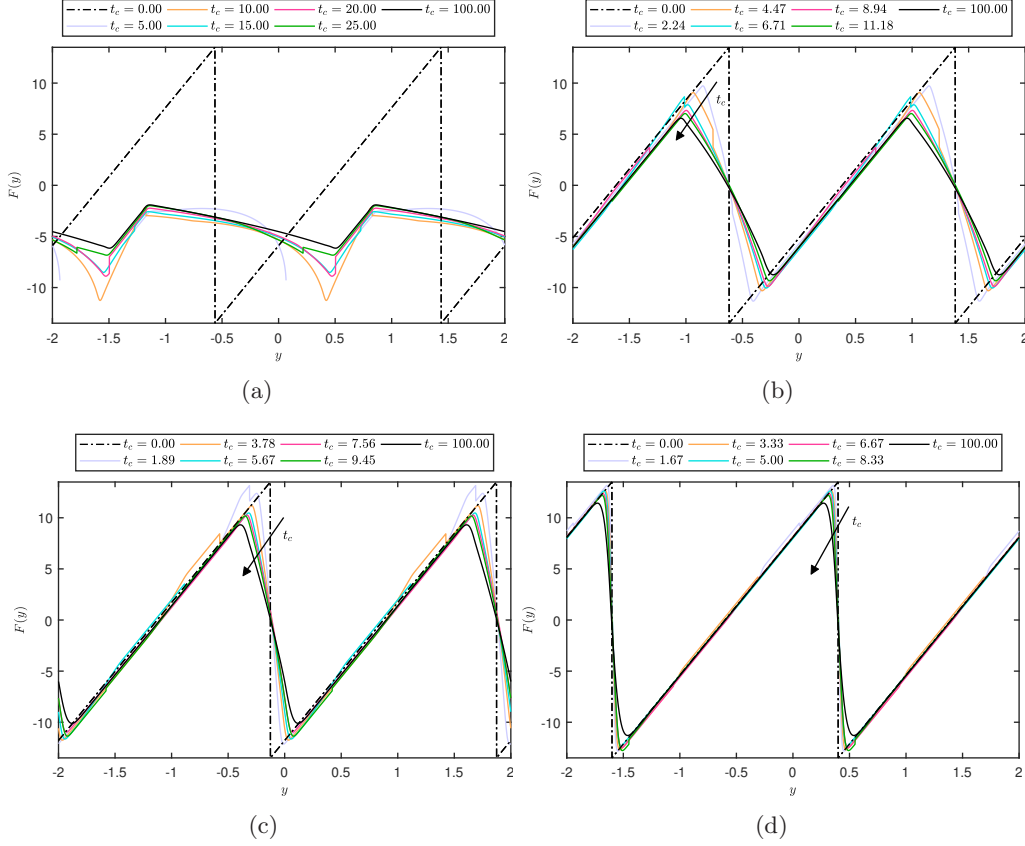


Figure 9: The waveform of the oscillation at long times, F , for barrier of heights (a) $D = 0.1$; (b) 0.5 ; (c), 0.7 ; and (d) 0.9 and for $w = 1$. In each figure, the waveform is extracted from the numerical simulations at seven comparison times t_c . For each case the waveforms converge to a single profile when t_c becomes sufficiently large, in accord with the asymptotic analysis. In (d) the waveform at late times is quite close to the waveform deduced from initial conditions ($t_c = 0$, dot-dashed line), but for (a)-(c) there are differences due to appreciable non-linear interactions during the initial phases of the motion.

and consequentially the simulation results are accurately represented by the asymptotic expressions.

Figure 9 shows the wave-form $F(y)$ as computed from the numerical simulations for a selection of barrier heights, D , and comparison times, t_c with $w = 1$. (The curve at $t_c = 0$ is given by (3.36) in each case.) For all cases, as time passes, the wave seen in the simulation converges on a time-independent shape, which is in accord with the asymptotic analysis. Indeed this confirms that the amplitude of the wave-like part of the dependent fields, h_w and u_w , decays as $(t - t_0)^{-3}$ and that the phase speed approaches \sqrt{D} (see (3.26c) and (3.26d)). In fact we observe for $\sqrt{D}(t_c - t_0) \gtrsim 10$ there is very little change in the computed shape of F , indicating that by that stage the numerical results

are accurately approximated by the asymptotic form. We now discuss the figures in order of increasing ϵ_t (see table 1), *i.e.* most accurate to least accurate.

Figure 9d shows that for relatively large values of D , the asymptotic expansion based upon $t_c = 0$ accurately predicts the numerically computed phase speed and the wave shape. Figures 9c and 9b, in contrast, reveal some discrepancies. Examining the portions with positive gradient (which correspond to uniform and simple wave regions in the hodograph solution) we see excellent correspondence between different times suggesting that we have both the correct phase speed and wave shape for these portions. However, the portions with negative gradient (corresponding to simple and complex wave regions) are not very accurate, and these are precisely the regions in which the non-linear wave self-interaction is important. Figure 9a reveals that for relatively small values of D , we do not have the correct wave phase speed for early times, because the simulated wave-forms drifts off of the predicted wave even in the positive gradient regions. Furthermore it is evident that the calculation of the waveform F from the initial conditions ($t_c = 0$) does not provide a useful indication of the behaviour at later times. Instead one must deduce the asymptotic form from the profiles at later times, t_c , or one must employ a higher order expansion, to predict the phase speed accurately.

4. Draining through a constriction

In this section we compute the unsteady drainage that occurs through a constriction at one end of the reservoir. In this situation, the drainage boundary condition is given by (2.3) with $D = 0$ and $0 < w \leq 1$, which may be written

$$w = Fr \left(\frac{3}{Fr^2 + 2} \right)^{3/2}. \quad (4.1)$$

Thus given a relative width of the constriction, w , we may determine the Froude number and this remains constant throughout the entire motion. We demonstrate below that many of the features of the unsteady flow over a barrier are reproduced for the current scenario of flows through constrictions, importantly including the successive nonlinear wave reflections from the backwall and drainage boundary. There are differences, however, in the evolution at long times; for flows through a constriction, the depth of the fluid layer ultimately vanishes and does so in a self-similar way, reflecting the absence of any additional lengthscales in the shallow water model of the motion. We construct the solutions for the motion using the hodograph transformation and the method of characteristics, numerical computations, and we derive and assess the linear stability of the similarity solution that governs the dynamics at late times.

4.1. Characteristic and hodograph methods

The draining boundary at $x = 1$ (4.1) imposes a constant Froude number on the flow at this location and leads to a considerable simplification of the solution derived for flow over a barrier. In the hodograph plane this boundary condition (4.1) is represented by

$$\beta_s = -\frac{(2 - Fr)}{(2 + Fr)}\alpha. \quad (4.2)$$

It is a straight line, rather than a curved segment as in §3, and terminates at the origin.

The solution features a similar pattern of uniform, simple and complex wave regions (see figures 2 and 10). The key value of the characteristic variable is $\beta = \beta_* = \beta_s(2) = -2(2 - Fr)/(2 + Fr)$ and the β -characteristics associated with this value bound the first

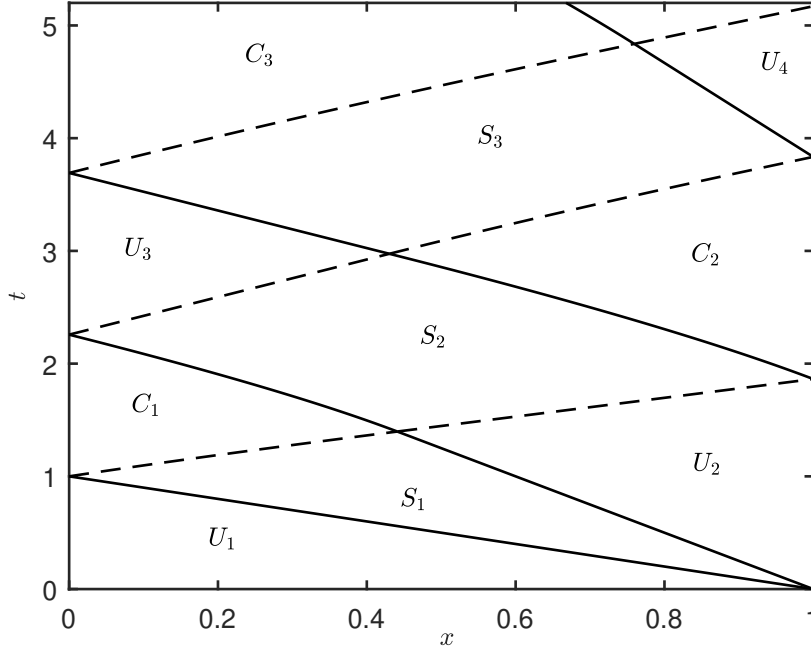


Figure 10: The characteristic plane for the unsteady draining of fluid from a partially confined reservoir with constriction width, $w = 4/3^{3/2}$ and Froude number $Fr = 0.5$. In this figure we plot and label the uniform, simple and complex wave regions following the initiation of the flow. We also plot the α - and β -characteristics with dashed and solid lines, respectively.

complex wave region attached to the back wall, C_1 and the first complex wave region attached to the drainage boundary. The construction of the solution for $t(\alpha, \beta)$ and $x(\alpha, \beta)$ in complex wave region C_1 is identical to that presented in §3 (see (3.4)). The integral equation for $t(\alpha, \beta)$ within region C_2 is, however, somewhat simpler than (3.14), because the boundary condition is enforced along a straight line in the hodograph plane. For a flow through a constriction with Froude number Fr , it is given by

$$\begin{aligned}
 t(\alpha, \beta_s) = & B(\alpha, \beta_*; \alpha, \beta_s) t_{b3}(\alpha) + B(2, \beta_*; \alpha, \beta_s) t_{b3}(2)^{\frac{1}{2}} (Fr - 1) \\
 & + B(\alpha_s, \beta_s; \alpha, \beta_s) t(\alpha_s, \beta_s)^{\frac{1}{2}} (1 - Fr) - \int_2^\alpha \left(\frac{\partial B}{\partial a} - \frac{3B}{2(a - \beta_*)} \right) t_{b3}(a) da \\
 & - \int_{\alpha_s}^2 \frac{t(a_s, b_s)}{2} \left((1 + Fr) \frac{\partial B}{\partial a} + \frac{(Fr - 1)(Fr - 2)}{(Fr + 2)} \frac{\partial B}{\partial b} - \frac{3Fr}{2a} B \right) da. \quad (4.3)
 \end{aligned}$$

As in §3, we evaluate this expression numerically by first computing $t_s(\alpha) = t(\alpha, \beta_s(\alpha))$, which is the t -field along the boundary ($x = 1$), before employing numerical quadrature to determine both $t(\alpha, \beta)$ and $x(\alpha, \beta)$. Numerical iteration is used to find the solution for $t(\alpha, \beta_s(\alpha))$ - we find that a converged solution to within a tolerance of 10^{-12} is typically found within 6 iterative steps. In the hodograph plane, the complex wave region C_2 forms a triangle given by $\{(\alpha, \beta) : \beta_* \leq \beta \leq \beta_s(\alpha), -\beta_* \leq \alpha \leq 2\}$ and we note that $\beta_m = \beta_s(-\beta_*) = 2[(2 - Fr)/(2 + Fr)]^2$.

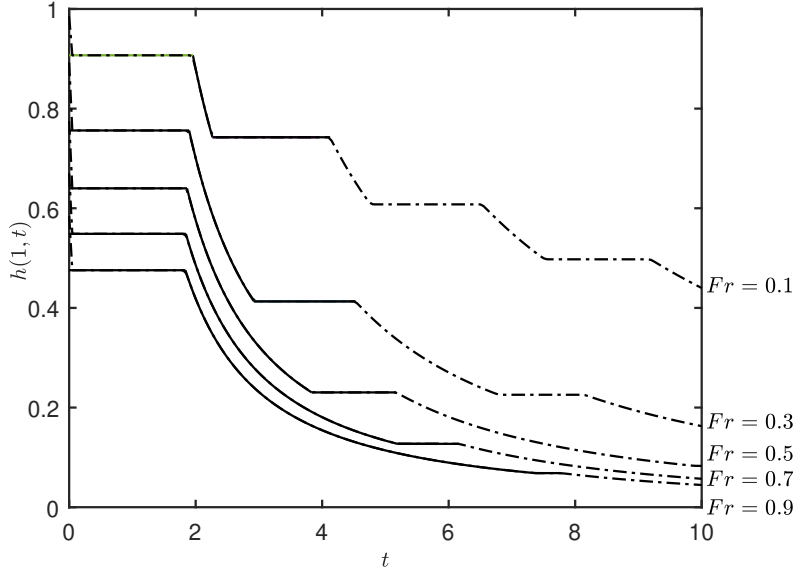


Figure 11: The height at the freely-draining boundary, $h(1, t)$, as a function of time for various values of the outflow Froude number, Fr (and consequentially varying constriction widths, w). The solid lines correspond to results from integration in the hodograph plane, where the dot-dashed lines correspond to numerical integration of the shallow water equations (2.1a)-(2.1b). The quasi-analytical hodograph calculations were performed until the end of region U_4 , but the numerical computations were continued over much longer times. We note the very close agreement between the two approaches, with the only very slight differences occurring at the junctions of the intervals during which the height is uniform or decreasing.

Having numerically evaluated the solution in region C_2 we can compute the α -characteristics, $x = x_{a2}$ and $x = x_{a3}$ on which $\alpha = -\beta_*$ (see figures 2 and 10). The latter bounds the complex wave region C_3 and using (3.19) we may evaluate the solution within it. These steps then complete the computation of the solution following the first three reflections from the boundaries.

We illustrate the solution by plotting the temporal variation of the height at the draining boundary, $h(1, t)$, for various values of the Froude number, Fr , and consequentially, the constricted width, w (see figure 11). We observe that the height at that location features periods during which it is constant, corresponding to the uniform regions in figure 10, and periods when it declines. We have computed the first three phases using quasi-analytical techniques described above; in principle these could be continued for later times, but in this study we compute the evolution using the direct numerical integration of the governing shallow water equations (2.1a)-(2.1b). We note the close correspondence between the two approaches, with the only apparent differences visible in figure 11 occurring when the solution changes from one wave region to another (and the temporal gradient of the height is discontinuous).

4.2. Long time evolution: similarity solution

After relatively long times, the flow evolves from its initial conditions to a state in which the motion is well-captured by similarity solutions that are constructed below. These extend the solution developed by Momen *et al.* (2017) for an unconstricted outflow, in which the Froude number is unity, to a more general case where the Froude number is given by (4.1). As shown by Momen *et al.* (2017), for this class of similarity solution, the spatial coordinate, x , is not geared to time and the height and velocity fields have the following form,

$$h(x, t) = \frac{\hat{H}(x)}{t^2} \quad \text{and} \quad u(x, t) = \frac{\hat{U}(x)}{t}, \quad (4.4)$$

where $\hat{H}(x)$ and $\hat{U}(x)$ are to be determined. This dependence on t is required in order to satisfy the drainage condition, $u(1, t) = Fr\sqrt{h(1, t)}$, and to balance the terms in the momentum equation (2.1*b*). On substitution into the governing equations (2.1*a*) & (2.1*b*), we find that

$$\frac{d\hat{H}}{dx} = \frac{\hat{U}\hat{H}}{\hat{U}^2 - \hat{H}} \quad \text{and} \quad \frac{d\hat{U}}{dx} = \frac{\hat{U}^2 - 2\hat{H}}{\hat{U}^2 - \hat{H}}, \quad (4.5)$$

together with the boundary conditions at the backwall and at the outflow, respectively given by

$$\hat{U}(0) = 0 \quad \text{and} \quad \hat{U}(1) = Fr\sqrt{H_1}, \quad (4.6)$$

where $H_1 = \hat{H}(1)$. We progress by introducing $V = \hat{U}^2/\hat{H}$ and then on the assumption that $d\hat{H}/dx$ does not vanish, we find that

$$\frac{dV}{d\hat{H}} = \frac{V - 4}{\hat{H}}. \quad (4.7)$$

Integrating (4.7), and imposing $\hat{H} = H_0$ when $V = \hat{U} = 0$ at $x = 0$, we deduce that

$$V = 4 \left(1 - \frac{\hat{H}}{H_0} \right). \quad (4.8)$$

The latter boundary condition (4.6) demands that $V = Fr^2$ when $\hat{H} = H_1$ at $x = 1$, and thus from (4.8), we find that

$$\frac{H_1}{H_0} = 1 - \left(\frac{Fr}{2} \right)^2. \quad (4.9)$$

To obtain $\hat{H}(x)$ we rewrite (4.8) in terms of \hat{U}

$$\hat{U} = 2\sqrt{\hat{H}}\sqrt{1 - \frac{\hat{H}}{H_0}} \quad (4.10)$$

and substitute into (4.5a) to derive the implicit solution

$$\frac{x}{\sqrt{H_0}} = -\frac{\pi}{2} + 2\sqrt{\frac{\hat{H}}{H_0}}\sqrt{1 - \frac{\hat{H}}{H_0}} + \arcsin\sqrt{\frac{\hat{H}}{H_0}}. \quad (4.11)$$

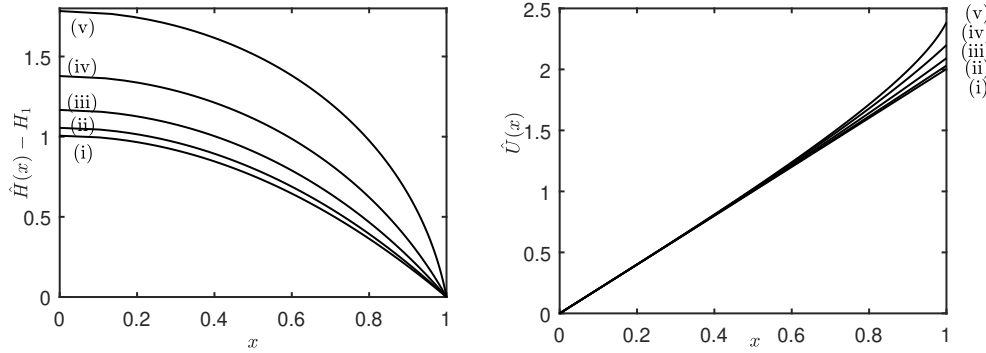


Figure 12: The similarity functions (a) $H(x) - H_1$ and (b) $U(x)$ as function distance from the back wall, x , for various Froude numbers at the constriction ($x = 1$): (i) $Fr = 0.1$; (ii) $Fr = 0.3$; (iii) $Fr = 0.5$; (iv) $Fr = 0.7$; and (v) $Fr = 0.9$.

The value of H_0 is found by setting $x = 1$ and substituting (4.9) into the above equation, which yields

$$H_0 = \left(-\frac{\pi}{2} + Fr \sqrt{1 - \left(\frac{Fr}{2} \right)^2} + \arccos \frac{Fr}{2} \right)^{-2}. \quad (4.12)$$

The velocity field $\hat{U}(x)$ is found from (4.10). The solutions for various values of the Froude number are plotted in figure 12. We note that the fluid decreases in depth monotonically from the rear of the flow to the front. This generates a streamwise pressure gradient that accelerates the flow and drives the fluid through the constriction. Lower values of the Froude number, corresponding to smaller widths at the constriction, lead to reduced velocities and weaker pressure gradients.

These similarity solutions are compared with the results from the numerical integration of the governing equations; in figure 13, we compare the numerically determined height at the outflow with the prediction from the similarity solution. In this study we have integrated numerically from lock-release initial conditions, which are evidently not in self-similar form of (4.4); thus we find that the similarity solutions is approached progressively. We also note that the self-similarity functions (4.4) are only defined up to an arbitrary origin in time; in other words, we could replace t with $t - \hat{t}_0$ in (4.4) and the same solutions are maintained. Evidently at sufficiently long times, $t \gg |\hat{t}_0|$, this makes negligible difference to the predictions of the similarity solutions, but at short times, we may select \hat{t}_0 to closely match the evolution from lock release conditions at relatively short times. In figure 13 we have chosen $\hat{t}_0 = -\sqrt{H_1}$, so that the similarity solution predicts a height of unity at $x = 1$ and $t = 0$ and we note that this leads to a close correspondence with the numerically computed value (see figure 13). For relatively small of the Froude number, Fr , the magnitude of the temporal offset, \hat{t}_0 is quite large; its effect is that the long time, self-similar form in which $h(1, t)$ is proportional to $1/t^2$ is not realised until very late times (and in the case $Fr = 0.1$, beyond the computational range plotted in figure 13).

The similarity solution captures the leading order decay of the dependent variables. However it does not include the oscillations that occur as fluid sloshes between the backwall and the drainage boundary. These oscillations are manifest as intervals during which the height at the drainage boundary remains constant (figure 13). Interestingly,

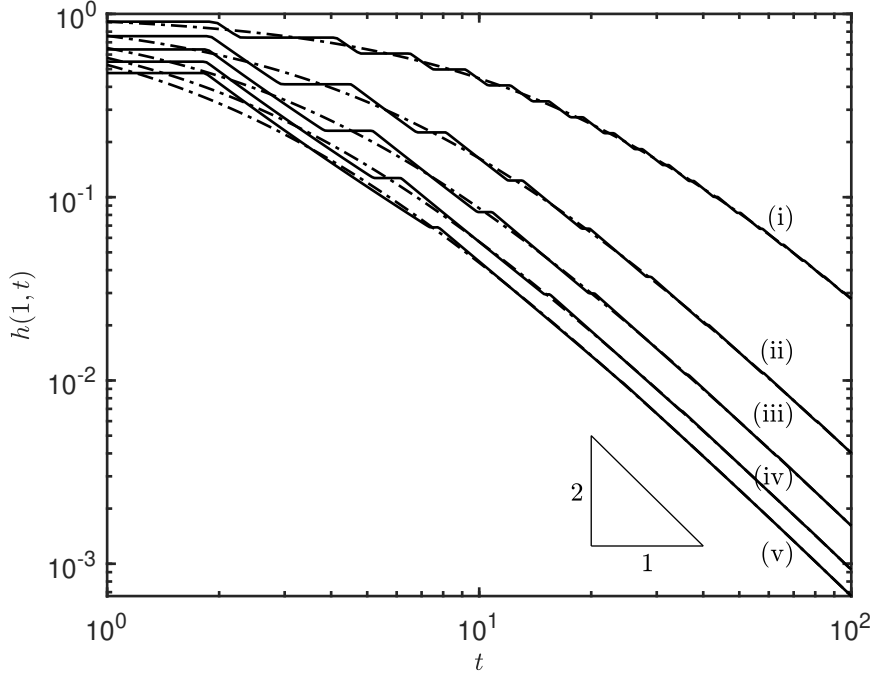


Figure 13: The height at the outflow, $h(1, t)$, as a function of time for various values of the Froude number: (i) $F = 0.1$; (ii) $F = 0.3$; (iii) $F = 0.5$; (iv) $F = 0.7$; and (v) $F = 0.9$. The solid line plots the results from the numerical integration of the governing equations initiated with lock-release conditions and the dotted lines plots the similarity solution, $H_1/(t + \sqrt{H_1})^2$.

and in contrast to the draining flow over a barrier §3, they appear not to have a fixed period, but rather the duration between them progressively increases. This observation will be studied in the next subsection.

4.3. Linear stability of similarity solution

We may analyse the progressive approach of the draining dynamics through a constriction to the similarity solution (4.4), by computing the linear stability of the latter. Following Grundy & Rottman (1985) and Mathunjwa & Hogg (2006), we introduce a small perturbation to the solutions for the height and velocity fields, denoted by $\tilde{H}(x)$ and $\tilde{U}(x)$ respectively, so that

$$h = \frac{\hat{H}(x)}{t^2} + \delta \frac{\tilde{H}(x)}{t^{\gamma+2}} \quad \text{and} \quad u = \frac{\hat{U}(x)}{t} + \delta \frac{\tilde{U}(x)}{t^{\gamma+1}}, \quad (4.13)$$

where δ is a small ordering parameter ($\delta \ll 1$) and γ is to be determined. On substitution into the governing equations (2.1a) & (2.1b) and after linearisation, we find that

$$-(2 + \gamma)\tilde{H} + \frac{d}{dx}(\tilde{H}\hat{U} + \tilde{U}\hat{H}) = 0, \quad (4.14)$$

$$-(1 + \gamma)\tilde{U} + \frac{d}{dx}(\tilde{U}\hat{U} + \tilde{H}) = 0. \quad (4.15)$$

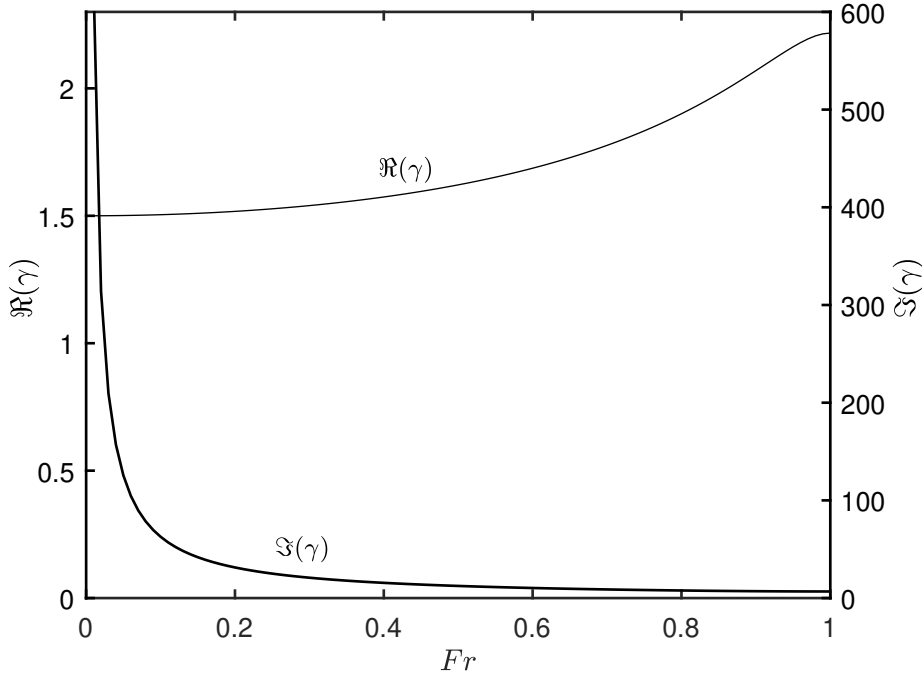


Figure 14: The exponent, γ , as a function of the Froude number, Fr .

The boundary conditions represent no flow at the backwall, $\tilde{U}(0) = 0$, and the linearised drainage condition is given by

$$2\hat{U}(1)\tilde{U}(1) = Fr^2\tilde{H}(1). \quad (4.16)$$

Straightforwardly, we note that $\gamma = 1$ is a solution, with $\tilde{H} = 2\hat{H}$ and $\tilde{U} = \hat{U}$; this corresponds to the introduction of a shift of the temporal origin, which, as discussed above, does not change the similarity solution. In addition, $\gamma = 0$ is a solution with $\tilde{U} = d\hat{U}/dx$ and $\tilde{H} = d\hat{H}/dx$; this corresponds to a translation of the x -axis. Both of these solutions are inevitably found in the study of the stability of similarity solutions and do not reveal the linear stability properties (Mathunjwa & Hogg 2006). Instead we seek non-trivial solutions to (4.14)-(4.15), which determines the exponent, γ , as an eigenvalue.

We determine the eigenvalue, γ , through numerical integration of (4.14)-(4.15), using the solution for $\hat{H}(x)$ and $\hat{U}(x)$ given by (4.10) and (4.11); in this computation, we allow the eigenvalue and functions to be complex-valued, noting that since the governing equations have real-valued coefficients, they are also satisfied by the complex conjugates of the solutions. We plot the value of γ with the smallest real part in figure 14 as a function of the drainage Froude number, Fr . We note that $\text{Re}(\gamma) > 0$ and thus the similarity solutions are linearly stable; in other words, all disturbances decay more rapidly than the solutions itself. Infact, we note that $\text{Re}(\gamma)$ is only relatively weakly dependent upon Fr . However the eigenvalue also has a non-vanishing imaginary part and its relatively strong variation is also plotted in figure 14.

We may use this solution to analyse the oscillations noted above in the numerically determined solution to the drainage from lock-release initial conditions (figure 13). Denoting the ratio between successive times at which the height at the end of the domain

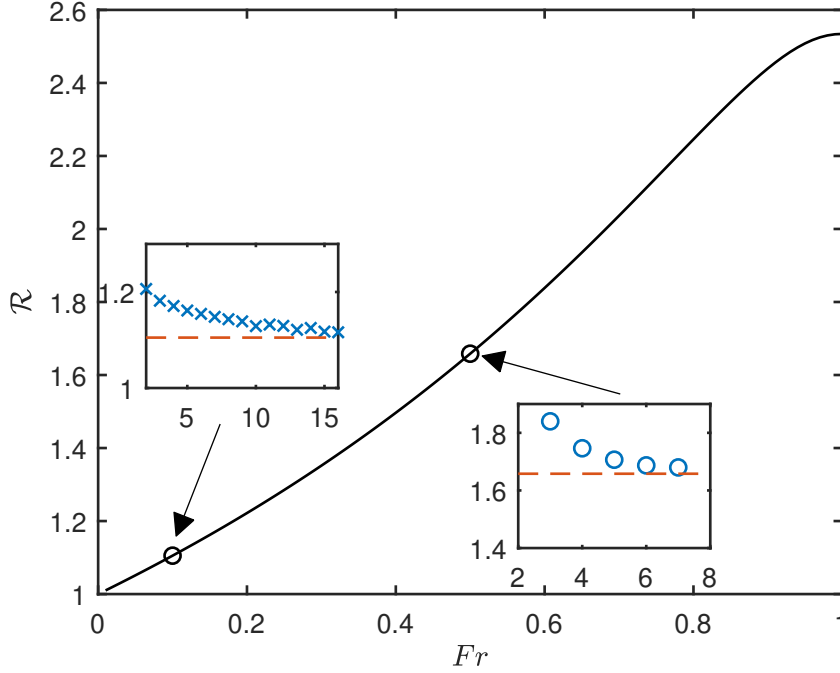


Figure 15: The ratio of successive times at which the height at the outflow exhibits periods during which it is constant, \mathcal{R} , as a function of the Froude number, Fr . The inset figures show the numerically determined ratio, \mathcal{R}_n , as a function of the oscillation number n (shown with symbols) and the theoretical prediction (dashed line) for $Fr = 0.1$ and $Fr = 0.5$.

exhibits a spatially uniform region by \mathcal{R} , we find that

$$\mathcal{R} = \exp\left(\frac{2\pi}{\gamma_I}\right) \quad (4.17)$$

where $\gamma_I \equiv \text{Im}(\gamma)$ denotes the imaginary part of the exponent γ . In figure 15 we plot \mathcal{R} as a function of the Froude number, Fr ; we observe that the ratio of successive periods increases monotonically with Fr . We also examine this ratio in our numerical results by computing $\mathcal{R}_n = (t_n - t_0)/(t_{n-1} - t_0)$, the ratio of the n^{th} to $(n-1)^{\text{th}}$ times at which the period of a constant fluid depth at $x = 1$ starts as a function of n for $Fr = 0.1$ and $Fr = 0.5$ (see inset figures in figure 15). In both cases we find that the numerically determined ratio approaches the asymptotic prediction; initially there are differences because the motion is strongly non-linear, but these diminish as the flow evolves into a state that is accurately represented by this linearised analysis. We note that for $Fr = 0.1$ there are many oscillations observed during the computational period, because the ratio, \mathcal{R} is quite close to unity. In contrast, when $Fr = 0.5$, the ratio of the successive oscillations is larger and so fewer can be determined in the numerical results before their amplitude has very substantially diminished.

5. Summary and conclusions

The fluid mechanics of the gravitationally-driven drainage of an initially quiescent reservoir following the partial removal of its confining dam have been investigated in detail using the shallow water equations to reveal the temporal and spatial dependencies of the depth and velocity of the flowing layer. For scenarios in which the reservoir drains over a barrier and through a constriction, we have shown how the unsteady motion evolves towards a state in which there is progressive drainage and relatively rapid wave motions that run between the backwall of the reservoir and the partially removed dam. When the fluid drains over a weir (a partially removed dam that leaves a barrier of height D), we find that after sufficient time following the flow initiation, the excess height diminishes in proportion to t^{-2} , while the velocity at the overflow varies as t^{-3} . Relatively fast moving waves propagate between the outflow and backwall of the reservoir with dimensionless phase speed, \sqrt{D} and amplitude that diminishes as t^{-3} . In contrast when the flow is through a constriction alone ($D = 0$ and $w < 1$), the height of the fluid layer also decreases as t^{-2} , but the velocity is proportional to t^{-1} . The motion in this scenario becomes self-similar to leading order after sufficient time has elapsed following initiation. Wave-like disturbances emerge, but their wave-speed diminishes exponentially in time and in relation to the width of the constricted outflow.

The results have been determined using three approaches, each of which has necessitated some innovations. First we have used the hodograph transformation to convert the governing non-linear partial differential equations to a linear system that may be tackled using analytical techniques. The solutions are given by quasi-analytical expressions and are evaluated through numerical quadrature to reveal the initial gravitational slumping and the onset of the oscillatory behaviour. In principle these calculations could be extended to much later times, although the oscillations entail rather tedious analytical bookkeeping. Direct numerical integration of the governing equations provide solutions as well and we have employed a numerical method that carefully imposes the boundary conditions at the outflow, while minimising internal numerical dissipation, so that features with discontinuous gradients of the dependent variables can be captured accurately. The numerical results, validated by the analytical results obtained through hodograph techniques, may be extended to late times to reveal the progressive drainage and the oscillations that manifest themselves as intervals during which the height and velocity of the fluid layer adjacent to the back wall or the outflow are spatially uniform. The oscillations persist although their amplitude diminishes. At late times for flows over a barrier, we have employed a multiple time-scale asymptotic technique to determine the draining and wave-like motions. The algebraic details of this asymptotic result are somewhat cumbersome given that the amplitude and phase are only established by calculating to relatively high order and using repeated secular conditions to ensure uniformity of the asymptotic expansion, but the results compactly reveal the oscillations within the reservoir. Furthermore the asymptotic solutions confirm the accuracy of the numerical computations at late times. In contrast, flows through constrictions approach a self-similar state at late times, in which the oscillations can be interpreted as the linear stability of this state. We showed that the direct prediction of the exponential growth of the period of the oscillations was realised by the numerical computations. The result and interpretation has bearing on other inertially-dominated, self-similar flows, such as gravity current dynamics from lock-release initial conditions.

The unsteady fluid mechanics of this study may have important consequences for the draining of natural-scale reservoirs following the partial removal of their containing dam. The calculations show that the volume flux of fluid per unit width leaving the reservoir

diminishes temporally and at late times is proportional to t^{-3} for both flows over weirs and through constrictions. It would be interesting to test this and other predictions from our analysis against laboratory or large-scale observations. The problem is also a useful test-bed solution for numerical integration of either the shallow-water equations, for which we have established a non-trivial analytical solution at both early and late times or for other simulations of fluid flow in this configuration. For example, the OpenFoam library for the direct numerical simulation of the Navier-Stokes equations identifies unsteady reservoir draining as an important trial problem and Momen *et al.* (2017) used this computational library to simulate flow over an edge ($D = 0, w = 1$); it would be also interesting to simulate flows for the more general problem analysed in this study. There are also several extensions to flows in related configurations that would be interesting to pursue. For example, how do the dynamics differ if the reservoir gravitationally slumps and flows towards a distant barrier, which it may overtop? Furthermore in the Boussinesq regime, how are the interactions modified if the impinging fluid is able to mix with its surroundings? Some aspects of this type of motion have been considered by Nabi & Flynn (2013), in which they model and confirm through experiments the mixing downstream of a weir. We anticipate that some of our new methodologies and results may be adapted to these new scenarios.

Acknowledgments EWGS acknowledges the support of an EPSRC (UK) studentship and AJH acknowledges the support of the Royal Society (APX\R1\180148).

Appendix A. Derivation of the outflow boundary condition

The outflow boundary condition at $x = 1$ can be found in textbooks such as Ackers *et al.* (1978) and Dake (1983), and is derived using classical hydrodynamic principles for inviscid flows. In particular for a steady state, the mass flux is constant and energy is conserved in regions where the flow varies continuously *i.e.* away from hydraulic jumps. Moreover as the fluid flows over a smooth barrier, or through a smooth constriction, it may accelerate from a subcritical state to a supercritical state and is therefore critical at some interior location.

The geometry of the barrier or constriction at the outflow is such that the variation in the width of the channel or the elevation of the bed induces non-negligible lateral and vertical velocities only over $1 - \hat{\epsilon} < x < 1$, where $\hat{\epsilon} \ll 1$ is a relatively small dimensionless length, whereas over the bulk of the interior ($0 \leq x \leq 1 - \hat{\epsilon}$), the bed elevation and width are constant and the flow is streamwise. We denote the width of the channel relative to the uniform width in the bulk by $\hat{w}(x)$, noting that $\hat{w}(1) = w$ at the narrowest part of the outflow. Likewise we denote the bed elevation by $\hat{D}(x)$, non-dimensionalised with respect to the initial depth of the reservoir and note that $\hat{D}(1) = D$. We may then extend our shallow water equations to include width variations; the governing equations (2.1a) and (2.1b) are then given by the dimensionless Saint-Venant equations (Chow 2009)

$$\frac{\partial}{\partial t}(\hat{w}h) + \frac{\partial}{\partial x}(u\hat{w}h) = 0, \quad \frac{\partial u}{\partial t} + \frac{\partial}{\partial x}\left(\frac{u^2}{2} + h + \hat{D}\right) = 0. \quad (\text{A } 1)$$

Respectively these represent the width- and depth-averaged expressions of the conservation of mass and the balance of streamwise momentum on the assumption that the contraction in width is much less than the streamwise extent of the reservoir ($W(1 - w)/L \ll 1$) so that the lateral velocities remain relatively small. We note that away from the outflow $0 \leq x \leq 1 - \hat{\epsilon}$, $\hat{w} = 1$ and $\hat{D} = 0$ and the governing equations reduce to (2.1a) and (2.1b). Furthermore the dimensionless extent of the upstream distance over

which the barrier or constriction influences the flow is estimated to be the greater of the ratio of the barrier height to the streamwise lengthscale, or the constricted width to the streamwise lengthscale, both of which are assumed to be small. The solutions within the region adjacent to the outflow ($1 - \hat{\epsilon} < x < 1$) evolve on very short timescales of $O(\hat{\epsilon})$, because this is the timescale over which characteristics cross the domain. Conversely within the bulk of the domain, the flow evolves on timescales of order unity. Therefore, for the purpose of modelling the flow in the bulk we may consider the flow local to the boundary to be in a quasi-steady state. In what follows we derive an outflow boundary condition that applies to the flow at $x = 1 - \hat{\epsilon}$, namely at the upstream location where the channel width and bed elevation induce non-streamwise velocities

The steady states of the Saint-Venant equations are given by

$$u\hat{w}h = q, \quad \frac{u^2}{2} + h + \hat{D} = E, \quad (\text{A } 2)$$

for some q and E , which represent the dimensionless volume flux and energy, respectively. The Saint-Venant equations provide a model for the evolution of a channel-averaged velocity; if, instead a three-dimensional velocity field had been modelled using the inviscid Euler equation, then (A 2b) with u^2 replaced by $|\mathbf{u}|^2$ would represent the application of Bernoulli's theorem on the surface streamline. In what follows we relate the flow conditions at the location where the channel begins to deviate from purely streamwise flow ($x = 1 - \hat{\epsilon}$) to conditions at the highest and narrowest point of the barrier ($x = 1$), at which position we assume that the flow is also only streamwise. Thus energy conservation between these two locations is given by (A 2b). We assume that the inflow at $x = 1 - \hat{\epsilon}$ is subcritical (slow and deep), and that in $x > 1$ the bed slopes downward and the channel widens so that the fluid becomes supercritical (fast and shallow). At $x = 1$, $E - D(x)$ is minimised and consequentially so is $u^2/2 + h$. Thus the flow is critical at the outflow $x = 1$, and we may write $u_{\text{out}} = \sqrt{h_{\text{out}}}$. Then by conservation of volume flux (A 2)

$$h_{\text{out}} = \left(\frac{q}{w}\right)^{2/3}, \quad (\text{A } 3)$$

which yields the expression for the energy

$$E = \frac{3}{2} \left(\frac{q}{w}\right)^{2/3} + D. \quad (\text{A } 4)$$

We construct the boundary condition for the bulk at $x = 1 - \hat{\epsilon}$ using conservation of energy ((A 2)b)

$$\frac{u^2}{2} + h \Big|_{x=1-\hat{\epsilon}} = \frac{3}{2} \left(\frac{q}{w}\right)^{2/3} + D, \quad \text{where} \quad q = wu_{\text{out}}h_{\text{out}} = uh|_{x=1-\hat{\epsilon}}. \quad (\text{A } 5)$$

Finally, limiting $\hat{\epsilon} \rightarrow 0$ yields the boundary condition

$$u^2 + 2(h - D) - 3 \left(\frac{uh}{w}\right)^{2/3} = 0 \quad \text{at} \quad x = 1. \quad (\text{A } 6)$$

The modelling framework in this appendix and in §2 is built upon the assumption that the depth of the fluid layer is much smaller than any streamwise or lateral lengthscale so that the pressure adopts a hydrostatic distribution. Deviations from this hierarchy of lengthscales could lead to headloss (dissipation) which is not included in the current description. If we assume that the energy dissipation is of magnitude ΔE between the

bulk and the outflow, then we have the condition

$$u^2 + 2(h - D) - 3 \left(\frac{uh}{w} \right)^{2/3} \Big|_{x=1} = 2\Delta E. \quad (\text{A } 7)$$

The energy loss, ΔE , has been evaluated empirically. Following Zachoval *et al.* (2014), we introduce a discharge coefficient C_d given by

$$uh = \left(\frac{2}{3} \right)^{3/2} C_d w \left(\frac{u^2}{2} + h - D \right)^{3/2}. \quad (\text{A } 8)$$

This means that the discharge coefficient measures the energy ratio between the critical outflow and the bulk, where the depths are measured relative to the bed elevation at the outflow, i.e.

$$C_d^{2/3} = \left[\frac{3}{2} \left(\frac{uh}{w} \right)^{2/3} \right] \Big/ \left[\frac{u^2}{2} + h - D \right] \Big|_{x=1}. \quad (\text{A } 9)$$

Note that the numerator in this expression is the energy at the outflow, despite being evaluated at the bottom of the slope, because the critical energy is only dependent on the volume flux (which is conserved between the bottom and top of the slope) and the width of the channel. It is clear from (A 9) that for energy conserving flows $C_d = 1$. Rearranging (A 9) using (A 7) we obtain

$$\Delta E = \frac{3}{2} \left(\frac{1}{C_d^{2/3}} - 1 \right) \left(\frac{uh}{w} \right)^{2/3}. \quad (\text{A } 10)$$

Thus we can write the boundary condition in terms of the discharge coefficient

$$u^2 + 2(h - D) - 3 \left(\frac{uh}{C_d w} \right)^{2/3} \Big|_{x=1} = 0. \quad (\text{A } 11)$$

From the results in Zachoval *et al.* (2014), we find that $0.83 \lesssim C_d \lesssim 0.95$, which gives us

$$1.03 \lesssim \frac{1}{C_d^{2/3}} \lesssim 1.13. \quad (\text{A } 12)$$

On this basis we conclude that the effect of dissipation can be neglected to a good approximation. Even if it cannot be ignored then we can simply include it in our model by modifying the definition of w to be the drainage coefficient multiplied by the ratio of outflow width to channel width.

Explicit account of viscous processes have been ignored in this analysis and potentially at late times, when the flow is very thin at the crest of the barrier, viscous stresses could play a dynamical role. To this end we assess when the dimensional inertial terms of the governing equations, $u\partial u/\partial x$, become comparable with the (currently omitted) dimensional viscous terms, $\nu\partial^2 u/\partial z^2$, where ν is the kinematic viscosity. The viscous stresses are largest where the flow is shallowest at the crest of the barrier, at which location the relevant streamwise dimensional lengthscale is $\hat{\epsilon}L$. Viscous terms therefore become non-negligible when the reduced Reynolds number, $Re \equiv \mathcal{U}\mathcal{H}^2/(\nu\hat{\epsilon}L)$, is order unity, where \mathcal{U} and \mathcal{H} are dimensional estimates of the velocity and height fields. Inserting our dimensionless expressions for the velocity and height at the outflow when the height of the fluid layer is comparable with the maximum barrier height, we find that

$$Re = \frac{(g'H^5)^{1/2}}{\hat{\epsilon}L\nu} (h - D)^{5/2}, \quad (\text{A } 13)$$

where the reduced gravity is denoted by $g' = \Delta\rho g/\rho$ and h and D are the dimensionless height of the fluid layer at the outflow of the domain and the barrier height, respectively. For free-surface flow of water $g' \sim 10\text{ms}^{-2}$ and $\nu \sim 10^{-6}\text{m}^2\text{s}^{-1}$. Then for a release of scale height $H \sim 10\text{m}$ and a barrier of streamwise lengthscale $\hat{\epsilon}L \sim 100\text{m}$, we find that the reduced Reynolds number becomes of order unity when $h - D \sim 10^{-3}$. Thus for these parameter values, the neglect of boundary viscous shear stress is also reasonable until $h - D \sim 10^{-3}$. Conversely at laboratory scales, $H \sim 10\text{cm}$ and $\hat{\epsilon}L \sim 1\text{cm}$, we find that that viscous effects are negligible until $h - D \sim 4 \times 10^{-2}$.

Appendix B. Computation of the Asymptotic Solution

B.1. Bounded Waves with Resonant Forcing

In order to construct the wave components in the multi-scale expansion of §3.3 we need to bound the terms in the expansions, producing solvability conditions that eliminate the secular terms. As we show in the following subsection, the possibility of secular terms comes from the existence of resonant forcing in the linearised wave equations. Our analysis often encounters a forced linear wave equation to which we apply homogeneous boundary conditions. Therefore we first present a general result that enables the construction of bounded solutions.

THEOREM 1. *Let $F_L, F_R, G_L, G_R : \mathbb{R} \rightarrow \mathbb{R}$ be such that the derivatives F'_L, F'_R, G'_L, G'_R have period 2. The solution $u(x, t)$ to the forced linear wave equation*

$$\frac{1}{4c^2} \left[\frac{\partial^2 u}{\partial t^2} - c^2 \frac{\partial^2 u}{\partial x^2} \right] = F'_L(ct + x) + F'_R(ct - x) + 2x[G''_L(ct + x) + G''_R(ct - x)], \quad (\text{B } 1)$$

on $x \in [0, 1]$ with homogeneous Dirichlet boundary conditions $u(0, t) = u(1, t) = 0$ is bounded if and only if

$$F_R(y) - F_L(y) + G_R(y) + G_L(y) + G'_R(y) - G'_L(y) = 0, \quad (\text{B } 2a)$$

$$F_R(y + 2) - F_R(y) = -[G_R(y + 2) - G_R(y)], \quad (\text{B } 2b)$$

$$F_L(y + 2) - F_L(y) = G_L(y + 2) - G_R(y), \quad (\text{B } 2c)$$

$$G'_R(y + 2) - G'_R(y) = 0, \quad (\text{B } 2d)$$

$$G'_L(y + 2) - G'_L(y) = 0, \quad (\text{B } 2e)$$

and u is bounded on some $(x, t) \in [0, 1] \otimes [t_0, t_0 + 2/c)$. Moreover, the solution can be expressed as

$$\begin{aligned} u(x, t) = & f(ct - x) - f(ct + x) + 2x[F_R(ct - x) + G_R(ct - x) - F_L(ct + x) + G_L(ct + x)] \\ & + 2x^2[G_R(ct - x) - G_L(ct + x)], \end{aligned} \quad (\text{B } 3)$$

where f is an arbitrary period 2 function.

Proof. We begin by simplifying the wave equation to the canonical form by making the change of variables $\xi = ct + x$, $\eta = ct - x$, and defining $U(\xi, \eta) = u((\xi - \eta)/2, (\xi + \eta)/2c)$, resulting in

$$\frac{\partial^2 U}{\partial \xi \partial \eta} = F'_L(\xi) + F'_R(\eta) + (\xi - \eta)[G''_L(\xi) + G''_R(\eta)] \quad (\text{B } 4)$$

which can be integrated to produce the general solution

$$U = \hat{f}(\eta) + \hat{g}(\xi) + \eta F_L(\xi) + \xi F_R(\eta) - \eta G_L(\xi) + \xi G_R(\eta) + \left(\xi\eta - \frac{\eta^2}{2}\right) G'_L(\xi) - \left(\xi\eta - \frac{\xi^2}{2}\right) G'_R(\eta), \quad (\text{B } 5)$$

where \hat{f} and \hat{g} are the arbitrary functions of integration. Imposing the boundary conditions $U = 0$ on $\eta = \xi$ and $\xi = \eta + 2$ yields

$$\hat{g}(\xi) = -\hat{f}(\xi) - \xi(F_L(\xi) + F_R(\xi) - G_L(\xi) + G_R(\xi)) - \frac{\xi^2}{2}(G'_L(\xi) - G'_R(\xi)), \quad (\text{B } 6a)$$

$$\begin{aligned} \hat{f}(\eta + 2) = & \hat{f}(\eta) - 2F_L(\eta + 2) + (\eta + 2)(F_R(\eta) - F_R(\eta + 2)) \\ & + 2G_L(\eta + 2) + (\eta + 2)(G_R(\eta) - G_R(\eta + 2)) - \\ & - 2G'_L(\eta + 2) + \left(2 - \frac{\eta^2}{2}\right) G'_R(\eta) + \frac{(\eta + 2)^2}{2} G'_R(\eta + 2), \end{aligned} \quad (\text{B } 6b)$$

respectively. Using these it is possible to show that

$$\begin{aligned} u(x, t + 2/c) - u(x, t) = & U(\xi + 2, \eta + 2) - U(\xi, \eta) \\ = & 2[F_L(\xi + 2) - F_R(\xi) - G_L(\xi + 2) - G_R(\xi) + G'_L(\xi + 2) - G'_R(\xi)] \\ & - 2[F_L(\eta + 2) - F_R(\eta) - G_L(\eta + 2) - G_R(\eta) + G'_L(\eta + 2) - G'_R(\eta)] \\ & + (\xi - \eta)[F_L(\xi) - F_L(\xi + 2) - G_L(\xi) + G_L(\xi + 2)] \\ & - (\xi - \eta)[F_R(\eta) - F_R(\eta + 2) + G_R(\eta) - G_R(\eta + 2)] \\ & + \frac{(\xi - \eta)^2}{2}[G'_L(\xi) - G'_L(\xi + 2)] - \frac{(\xi - \eta)^2}{2}[G'_R(\xi) - G'_R(\xi + 2)]. \end{aligned} \quad (\text{B } 7)$$

From here the conditions (B 2) follow. To deduce the form of the general solution (B 3) we make the substitution

$$f(y) = \hat{f}(y) + y(F_L(y) - G_L(y) + G'_L(y) - G'_R(y)) - \frac{y^2}{2} G'_R(y), \quad (\text{B } 8)$$

so that (B 6b) enforces $f(y + 2) = f(y)$, and then substitute (B 6a) into (B 5) and simplify using (B 2).

B.2. Expansion with Multiple Time Scales

We expand the depth and velocity as sequences of undetermined functions dependent on the time-scales (3.24)

$$h(x, t) - D = \epsilon^2 h_2(x, T_0, T_1) + \epsilon^3 h_3(x, T_0, T_1) + \dots, \quad (\text{B } 9a)$$

$$u(x, t) = \epsilon^3 u_3(x, T_0, T_1) + \epsilon^4 u_4(x, T_0, T_1) + \dots \quad (\text{B } 9b)$$

where we require that h_m and u_m are of order unity so that this is a uniform expansion. Substituting our expansions into the governing equations we obtain

$$0 = \frac{\partial h}{\partial t} + \frac{\partial (uh)}{\partial x} \quad (\text{B } 10)$$

$$\begin{aligned} &= \epsilon^2 \frac{d\omega_0}{dT_1} \frac{\partial h_2}{\partial T_0} + \epsilon^3 \left[\frac{d\omega_0}{dT_1} \frac{\partial h_3}{\partial T_0} + \frac{\partial h_2}{\partial T_1} + D \frac{\partial u_3}{\partial x} \right] \\ &+ \epsilon^4 \left[\frac{d\omega_0}{dT_1} \frac{\partial h_4}{\partial T_0} + \frac{\partial h_3}{\partial T_1} + \frac{d\omega_2}{dT_1} \frac{\partial h_2}{\partial T_0} + D \frac{\partial u_4}{\partial x} \right] \\ &+ \epsilon^5 \left[\frac{d\omega_0}{dT_1} \frac{\partial h_5}{\partial T_0} + \frac{\partial h_4}{\partial T_1} + \frac{d\omega_2}{dT_1} \frac{\partial h_3}{\partial T_0} + \frac{d\omega_3}{dT_1} \frac{\partial h_2}{\partial T_0} + D \frac{\partial u_5}{\partial x} + \frac{\partial (u_3 h_2)}{\partial x} \right] + O(\epsilon^6), \end{aligned} \quad (\text{B } 11)$$

$$0 = \frac{1}{D} \left(\frac{\partial (uh)}{\partial t} + \frac{\partial (u^2 h)}{\partial x} + h \frac{\partial h}{\partial x} \right) \quad (\text{B } 12)$$

$$\begin{aligned} &= \epsilon^2 \frac{\partial h_2}{\partial x} + \epsilon^3 \left[\frac{d\omega_0}{dT_1} \frac{\partial u_3}{\partial T_0} + \frac{\partial h_3}{\partial x} \right] + \epsilon^4 \left[\frac{d\omega_0}{dT_1} \frac{\partial u_4}{\partial T_0} + \frac{\partial u_3}{\partial T_1} + \frac{\partial h_4}{\partial x} + \frac{h_2}{D} \frac{\partial h_2}{\partial x} \right] \\ &+ \epsilon^5 \left[\frac{d\omega_0}{dT_1} \frac{\partial u_5}{\partial T_0} + \frac{\partial u_4}{\partial T_1} + \frac{d\omega_2}{dT_1} \frac{\partial u_3}{\partial T_0} + \frac{1}{D} \frac{d\omega_0}{dT_1} \frac{\partial (u_3 h_2)}{\partial T_0} + \frac{\partial h_5}{\partial x} + \frac{1}{D} \frac{\partial (h_2 h_3)}{\partial x} \right] + O(\epsilon^6). \end{aligned} \quad (\text{B } 13)$$

The boundary condition at the rear of the lock demands that u_m vanishes at $x = 0$, while the boundary condition at $x = 1$ yields

$$0 = \frac{u^2}{2} + (h - D) - \frac{3}{2} \left(\frac{uh}{w} \right)^{2/3} \quad (\text{B } 14)$$

$$\begin{aligned} &= \epsilon^2 \left[h_2 - \frac{3}{2} \left(\frac{Du_3}{w} \right)^{2/3} \right] + \epsilon^3 \left[h_3 - \left(\frac{Du_3}{w} \right)^{2/3} \frac{u_4}{u_3} \right] \\ &+ \epsilon^4 \left[h_4 - \left(\frac{Du_3}{w} \right)^{2/3} \left(\frac{u_5}{u_3} + \frac{h_2}{D} - \frac{u_4^2}{6u_3^2} \right) \right] + O(\epsilon^5). \end{aligned} \quad (\text{B } 15)$$

We observe that the governing equations at $O(\epsilon^2)$ give $h_2 \equiv h_2(T_1)$. The balances at higher order will result in inhomogeneous resonantly forced wave equations for u_m with inhomogeneous boundary conditions, which we manipulate into the form stated in Theorem 1. To do this we must homogenise the boundary condition at $x = 1$, which at every order is an equation of the form $u_m = 2f(cT_0 + 1, T_1)$ where c is the wave speed and f is a function of period 2 ($f(y + 2, T_1) = f(y, T_1)$) given in terms of functions from the expansion at lower order. We then make the substitution $\hat{u}_m = u_m - x[f(cT_0 + x, T_1) + f(cT_0 - x, T_1)] - x(1 - x)g(x)$ where the first term homogenises the boundary condition, and the second term eliminates any functions from the source terms of the wave equation that cannot be represented by Theorem 1. From this point on, the process of constructing the solution amounts applying the boundedness conditions from the theorem as solvability conditions; we find that once (B 2a) has been enforced then (B 2b) to (B 2e) will automatically be satisfied, and then using u_m to find h_m which will yield a further unknown function and solvability condition.

B.2.1. Leading Order Balance

At $O(\epsilon^3)$ the governing equations are given by

$$\frac{d\omega_0}{dT_1} \frac{\partial h_3}{\partial T_0} + D \frac{\partial u_3}{\partial x} = -\frac{\partial h_2}{\partial T_1} \quad \text{and} \quad \frac{d\omega_0}{dT_1} \frac{\partial u_3}{\partial T_0} + \frac{\partial h_3}{\partial x} = 0, \quad (\text{B } 16)$$

while the boundary conditions at this order are given by

$$u_3 = 0 \quad \text{at} \quad x = 0 \quad \text{and} \quad u_3 = \frac{w}{D} \left(\frac{2h_2}{3} \right)^{3/2} \quad \text{at} \quad x = 1. \quad (\text{B } 17)$$

We define

$$\hat{u}_3 = u_3 - \frac{w}{D} \left(\frac{2h_2}{3} \right)^{3/2} x, \quad (\text{B } 18)$$

so that \hat{u}_3 satisfies the linerised wave equation

$$\frac{\partial^2 \hat{u}_3}{\partial T_0^2} - c^2 \frac{\partial^2 \hat{u}_3}{\partial x^2} = 0 \quad (\text{B } 19)$$

where the wave speed $c(T_1) = \sqrt{D}/(d\omega_0/dT_1)$ and there are homogeneous boundary conditions $\hat{u}_3 = 0$ at $x = 0$ and 1 . Solving this system yields

$$u_3 = F_3(cT_0 - x, T_1) - F_3(cT_0 + x, T_1) + \frac{w}{D} \left(\frac{2h_2}{3} \right)^{3/2} x \quad (\text{B } 20)$$

where $F_3(y + 2, \dots) = F_3(y, \dots)$. Substituting into (B 16) yields

$$h_3 = \sqrt{D} [F_3(cT_0 - x, T_1) + F_3(cT_0 + x, T_1)] + H_3(T_0, T_1) \quad (\text{B } 21)$$

and then substituting both h_3 and u_3 into (B 16) and integrating gives

$$H_3 = - \left(\frac{\partial h_2}{\partial T_1} + w \left(\frac{2h_2}{3} \right)^{3/2} \right) \left(\frac{d\omega_0}{dT_1} \right)^{-1} T_0 + A_3(T_1). \quad (\text{B } 22)$$

The function A_3 can be absorbed into F_3 , so without loss of generality we take $A_3 = 0$. For h_3 to be bounded in T_0 , we deduce that H_3 must vanish. This solvability condition leads to

$$h_2 = \frac{3^3}{2w^2(T_1 - \tau)^2}. \quad (\text{B } 23)$$

Thus at $O(\epsilon^3)$ we have found that

$$h_2 = \frac{27}{2w^2(T_1 - \tau)^2}, \quad (\text{B } 24a)$$

$$h_3 = \sqrt{D} [F_3(cT_0 - x, T_1) + F_3(cT_0 + x, T_1)], \quad (\text{B } 24b)$$

$$u_3 = F_3(cT_0 - x, T_1) - F_3(cT_0 + x, T_1) + \frac{27x}{Dw^2(T_1 - \tau)^3}. \quad (\text{B } 24c)$$

B.2.2. Second Order Balance

At $O(\epsilon^4)$ the governing equations are given by

$$\frac{d\omega_0}{dT_1} \frac{\partial h_4}{\partial T_0} + D \frac{\partial u_4}{\partial x} = -\frac{\partial h_3}{\partial T_1} \quad \text{and} \quad \frac{d\omega_0}{dT_1} \frac{\partial u_4}{\partial T_0} + \frac{\partial h_4}{\partial x} = -\frac{\partial u_3}{\partial T_1}. \quad (\text{B } 25)$$

The boundary conditions at this order are given by

$$u_4 = 0 \quad \text{at} \quad x = 0 \quad \text{and} \quad u_4 = h_3 u_3^{1/3} \left(\frac{w}{D} \right)^{2/3} \quad \text{at} \quad x = 1. \quad (\text{B } 26)$$

We convert this system into a forced linear wave equation with homogeneous boundary conditions by introducing

$$\hat{u}_4 = u_4 - \frac{3h_3 x}{D(T_1 - \tau)} \quad (\text{B } 27)$$

and this function satisfies the equation

$$\frac{\partial^2 \hat{u}_4}{\partial T_0^2} - c^2 \frac{\partial^2 \hat{u}_4}{\partial x^2} = 2 \frac{\partial h_3}{\partial T_1} x + \frac{6}{T_1 - \tau} \frac{c^2}{D} \frac{\partial h_3}{\partial x} \quad (\text{B } 28)$$

$$\begin{aligned} &= 2\sqrt{D}T_0 \frac{dc}{dT_1} \left[-\frac{\partial^2 F_3}{\partial y^2}(cT_0 - x, T_1) + \frac{\partial^2 F_3}{\partial y^2}(cT_0 + x, T_1) \right] \\ &\quad + 2\sqrt{D} \left[-\frac{\partial F_3}{\partial y}T_1(cT_0 - x, T_1) + \frac{\partial F_3}{\partial y}T_1(cT_0 + x, T_1) \right] \\ &\quad + \frac{6\sqrt{D}}{T_1 - \tau} \frac{c^2}{D} \left[-\frac{\partial F_3}{\partial y}(cT_0 - x, T_1) + \frac{\partial F_3}{\partial y}(cT_0 + x, T_1) \right] \end{aligned} \quad (\text{B } 29)$$

subject to boundary conditions $\hat{u}_4 = 0$ at $x = 0$ and 1 . Having a non-vanishing source term proportional to T_0 will cause u_4 to become unbounded. Supposing that F_3 not constant (and so we do have waves), this means that $dc/dT_1 = 0$ thus $\omega_0 = T_1 + \phi$ for some constant ϕ (which acts as a phase angle), making a choice of pre-factor. Equation (B 28) is now in the form required for Theorem 1 with

$$F_L = -F_R = 2\sqrt{D} \left(\frac{\partial F_3}{\partial T_1} + \frac{3F_3}{T_1 - \tau} + \frac{1}{(T_1 - \tau)^3} \frac{dB_3}{dT_1} \right) + G_L, \quad (\text{B } 30)$$

and $G_L = G_R$ arbitrary functions of T_1 , $B_3(T_1)$ being an appropriately chosen arbitrary function introduced by the integration of $\partial F_L / \partial y$. Therefore for u_4 to be bounded we require $F_L = G_L$, which implies that

$$F_3(y, T_1) = \frac{\hat{F}_3(y) + B_3(T_1)}{(T_1 - \tau)^3} \quad (\text{B } 31)$$

where $\hat{F}_3(y+2) = \hat{F}_3(y)$. Importantly this shows that the amplitude of the waves decays with the inverse cubic power of time. This substitution also causes the right hand side of (B 28) to vanish, and therefore

$$u_4 = F_4(\sqrt{D}T_0 - x, T_1) - F_4(\sqrt{D}T_0 + x, T_1) + \frac{3h_3 x}{D(T_1 - \tau)} \quad (\text{B } 32)$$

where $F_4(y+2, T_1) = F_4(y, T_1)$. Substituting into (B 25) yields

$$h_4 = \sqrt{D} \left[F_4(\sqrt{D}T_0 - x, T_1) + F_4(\sqrt{D}T_0 + x, T_1) \right] + \frac{3u_3 x}{T_1 - \tau} - \frac{81x^2}{2Dw^2(T_1 - \tau)^4} + H_4(T_0, T_1) \quad (\text{B } 33)$$

and both h_4 and u_4 into (B 25) gives

$$\frac{\partial H_4}{\partial T_0} = \frac{-1}{(T_1 - \tau)^3} \frac{dB_3}{dT_1}, \quad \text{and therefore} \quad H_4 = \frac{-T_0}{(T_1 - \tau)^3} \frac{dB_3}{dT_1} + A_4(T_1). \quad (\text{B } 34)$$

Since H_4 must remain bounded, we therefore deduce

$$\frac{dB_3}{dT_1} = 0 \quad \text{and} \quad H_4 = A_4(T_1). \quad (\text{B } 35)$$

The functions B_3 and A_4 can be absorbed into \hat{F}_3 and F_4 respectively, so without loss of generality we set both to zero. Thus at this order we have deduced

$$h_2 = \frac{27}{2w^2(T_1 - \tau)^2}, \quad (\text{B } 36a)$$

$$h_3 = \frac{\sqrt{D}}{(T_1 - \tau)^3} \left[\hat{F}_3(\sqrt{D}T_0 - x) + \hat{F}_3(\sqrt{D}T_0 + x) \right], \quad (\text{B } 36b)$$

$$u_3 = \frac{1}{(T_1 - \tau)^3} \left[\hat{F}_3(\sqrt{D}T_0 - x) - \hat{F}_3(\sqrt{D}T_0 + x) + \frac{27x}{Dw^2} \right], \quad (\text{B } 36c)$$

$$h_4 = \sqrt{D} \left[F_4(\sqrt{D}T_0 - x, T_1) + F_4(\sqrt{D}T_0 + x, T_1) \right] + \frac{3u_3x}{T_1 - \tau} - \frac{81x^2}{2Dw^2(T_1 - \tau)^4} \quad (\text{B } 36d)$$

$$u_4 = F_4(\sqrt{D}T_0 - x, T_1) - F_4(\sqrt{D}T_0 + x, T_1) + \frac{3h_3x}{D(T_1 - \tau)}. \quad (\text{B } 36e)$$

B.2.3. Third Order Balance, Solvability Conditions Only

At $O(\epsilon^5)$ the governing equations are given by

$$\frac{\partial h_5}{\partial T_0} + D \frac{\partial u_5}{\partial x} = -\frac{\partial h_4}{\partial T_1} - \frac{d\omega_2}{dT_1} \frac{\partial h_3}{\partial T_0} - h_2 \frac{\partial u_3}{\partial x} \quad \text{and} \quad \frac{\partial u_5}{\partial T_0} + \frac{\partial h_5}{\partial x} = -\frac{\partial u_4}{\partial T_1} - \frac{d\omega_2}{dT_1} \frac{\partial u_3}{\partial T_0}, \quad (\text{B } 37)$$

and the boundary conditions are given by

$$u_5 = 0 \quad \text{at} \quad x = 0 \quad \text{and} \quad u_5 = \left(\frac{w}{D} \right)^{2/3} u_3^{1/3} h_4 + \frac{u_4^2}{6u_3} - \frac{h_2 u_3}{D} \quad \text{at} \quad x = 1. \quad (\text{B } 38)$$

Our intention at this order is not to find explicit solutions for h_5 and u_5 , but rather to use solvability conditions to deduce the leading order correct to the phase speed, ω_2 and the relationship between F_4 and \hat{F}_3 . First we convert this system to a forced linear wave equation subject to homogeneous boundary conditions by substituting

$$\begin{aligned} \hat{u}_5 = u_5 - & \left(\frac{3^5}{2D^2w^4} \frac{w^2 - 3}{(T_1 - \tau)^5} + \frac{w^2}{3^2(T_1 - \tau)^5} \left[\hat{F}_3(\sqrt{D}T_0 - x)^2 + \hat{F}_3(\sqrt{D}T_0 + x)^2 \right] \right. \\ & + \frac{3}{\sqrt{D}(T_1 - \tau)} \left[F_4(\sqrt{D}T_0 - x, T_1) + F_4(\sqrt{D}T_0 + x, T_1) \right] \Big) x \\ & + x(x^2 - 1) \frac{54}{D^2w^2(T_1 - \tau)^5}. \end{aligned} \quad (\text{B } 39)$$

The system for \hat{u}_5 is in the form required to apply Theorem 1, with $F_R = -F_L$, $G_L = G_R$, and

$$F_L = G_L + \frac{1}{2\sqrt{D}} \frac{\partial F_4}{\partial T_1} + \frac{3F_4}{2\sqrt{D}(T_1 - \tau)} - \frac{27}{8Dw^2(T_1 - \tau)^5} \frac{d\hat{F}_3}{dy} + \frac{w^2 \hat{F}_3^2}{18(T_1 - \tau)^5} \\ + \frac{1}{2(T_1 - \tau)^3} \frac{d\omega_2}{dT_1} \frac{d\hat{F}_3}{dy} - \frac{1}{2\sqrt{D}(T_1 - \tau)^3} \frac{dB_4}{dT_1}, \quad (\text{B } 40)$$

$$\frac{\partial G_L}{\partial y} = \frac{3\hat{F}_3}{D(T_1 - \tau)^5}, \quad (\text{B } 41)$$

where $B_4(T_1)$ is an appropriately chosen arbitrary function introduced by the integration of $\partial F_L / \partial y$. The theorem gives us that, for u_5 to be bounded, we require $F_L = G_L$. Solving (B 40) as a first order linear differential equation in F_4 produces

$$F_4 = \frac{\sqrt{D}w^2 \hat{F}_3^2}{9(T_1 - \tau)^4} - \left[\frac{27}{4\sqrt{D}w^2(T_1 - \tau)^4} + \frac{\sqrt{D}\omega_2}{(T_1 - \tau)^3} \right] \frac{d\hat{F}_3}{dy} + \frac{\hat{G}_4 + B_4}{(T_1 - \tau)^3}. \quad (\text{B } 42)$$

where $\hat{G}_4(y+2) = \hat{G}_4(y)$. We observe that \hat{F}_3 may not be continuous, and its derivatives may not be bounded, so as a solvability condition we enforce that its coefficient must be zero, that is

$$\omega_2 = \frac{-27}{4Dw^2(T_1 - \tau)}. \quad (\text{B } 43)$$

This expression determines the leading order correction to the phase speed. The function \hat{G}_4 cannot impact the expansion, since it must be bounded, and when we reassemble h and u and limit $\epsilon \rightarrow 0$, $\epsilon \hat{G}_4 \rightarrow 0$. Therefore, without loss of generality, we take $\hat{G}_4 = 0$, thus

$$F_4 = \frac{\sqrt{D}w^2 \hat{F}_3^2}{9(T_1 - \tau)^4} + \frac{B_4}{(T_1 - \tau)^3}. \quad (\text{B } 44)$$

Constructing u_5 and h_5 and considering the additional requirements such that h_5 is bounded produces the condition

$$\frac{dB_4}{dT_1} = -\frac{3^3 \cdot 5}{4D^{3/2}w^2(T_1 - \tau)^2} \quad \text{and therefore} \quad B_4 = \frac{3^3 \cdot 5}{4D^{3/2}w^2(T_1 - \tau)} + \hat{B}_4. \quad (\text{B } 45)$$

Similarly to \hat{G}_4 we can set $\hat{B}_4 = 0$. Thus the expansion is then given by

$$h_2 = \frac{27}{2w^2(T_1 - \tau)^2}, \quad (\text{B } 46a)$$

$$h_3 = \frac{\sqrt{D}}{(T_1 - \tau)^3} \left[\hat{F}_3(\sqrt{D}T_0 - x) + \hat{F}_3(\sqrt{D}T_0 + x) \right], \quad (\text{B } 46b)$$

$$u_3 = \frac{1}{(T_1 - \tau)^3} \left[\hat{F}_3(\sqrt{D}T_0 - x) - \hat{F}_3(\sqrt{D}T_0 + x) + \frac{27x}{Dw^2} \right], \quad (\text{B } 46c)$$

$$h_4 = \frac{1}{(T_1 - \tau)^4} \left[\frac{Dw^2}{9} \left(\hat{F}_3(\sqrt{D}T_0 - x)^2 + \hat{F}_3(\sqrt{D}T_0 + x)^2 \right) + 3x \left(\hat{F}_3(\sqrt{D}T_0 - x) - \hat{F}_3(\sqrt{D}T_0 + x) \right) + \frac{27(3x^2 + 5)}{2Dw^2} \right] \quad (\text{B } 46d)$$

$$u_4 = \frac{1}{(T_1 - \tau)^4} \left[\frac{\sqrt{D}w^2}{9} \left(\hat{F}_3(\sqrt{D}T_0 - x)^2 - \hat{F}_3(\sqrt{D}T_0 + x)^2 \right) + \frac{3x}{\sqrt{D}} \left(\hat{F}_3(\sqrt{D}T_0 - x) + \hat{F}_3(\sqrt{D}T_0 + x) \right) \right] \quad (\text{B } 46e)$$

As a final observation, if we make the substitutions

$$\tau = \tilde{\tau} + \epsilon k, \quad \text{and} \quad \hat{F}_3 = \tilde{F}_3 + \frac{27k}{2\sqrt{D}w^2}, \quad (\text{B } 47)$$

and expand our asymptotic results using

$$\frac{1}{(T_1 - \tau)^p} = \frac{1}{(T_1 - \tilde{\tau})^p} \frac{1}{(1 - \epsilon k / (T_1 - \tilde{\tau}))^p} = \frac{1}{(T_1 - \tilde{\tau})^p} + \epsilon \frac{pk}{(T_1 - \tilde{\tau})^{p+1}} + \epsilon^2 \frac{p(p+1)k^2}{2(T_1 - \tilde{\tau})^{p+2}} + \dots \quad (\text{B } 48)$$

we find that on collection of terms with the same powers of ϵ , that the effect on the expansion is identical to that of the direct replacement τ with $\tilde{\tau}$ and \hat{F}_3 with \tilde{F}_3 . The solution is invariant, therefore, with respect to (B 47), and we are free to choose τ such that

$$\int_{y_0}^{y_0+2} \hat{F}_3(y) dy \quad (\text{B } 49)$$

takes any constant value. In particular, forcing this integral to vanish leads to a condition that determines the temporal offset, τ , as described in §3.3.

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